

Government of Nepal National Reconstruction Authority

Singhadurbar, Kathmandu



For earthquake damaged houses that needs to be repaired and retrofitted under HOUSING RECONSTRUCTION PROGRAMME

2017

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REPAIR AND RETROFITTING MANUAL for MASONRY STRUCTURE

For earthquake damaged houses that needs to be repaired and retrofitted under HOUSING RECONSTRUCTION PROGRAMME



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FOREWARD



I would sincerely like to congratulate everyone involved in the development of the "Repair and Retrofitting Manual for Masonry Structures" which has been published by the National Reconstruction Authority. This manual will support the implementation of the 100,000 NPRs retrofitting grant for partially damage houses that need seismic retrofitting under the GoN housing reconstruction programme.

Thirty-one districts have been identified by the GoN Post Disaster Needs Assessment (PDNA) as being earthquake affected. To date, almost 25,000 households across these districts have been identified as eligible to receive the 100,000 NPRs housing retrofit grant. The grant will be disbursed in two tranches based on compliant construction.

Every effort is required to support households to retrofit unsafe structures so that they can receive the grant amount. This manual has been developed for technical staff to support them to guide households through the retrofit process, and to manage the inspections of completed retrofits.

I look forward to seeing the manual implemented across the earthquake affected districts and to seeing the impact that it will have. This represents another positive step forward in the reconstruction process, and will support households to retrofit their home so that it is safe, compliant, and resilient in the face of future disasters.

> Dr. Govind Raj Pokharel Chief Executive Officer, NRA

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PREFACE



Under the housing reconstruction programme, houses that are partially damaged, and fall under damage grade 2 (major) and 3 (minor) are eligible to receive a 100,000 NPRs grant to retrofit their home. For the household to receive the housing retrofit grant, their home must comply with all the specifications detailed in the inspection check sheet, which were formulated based on the Minimum Requirements (MRs). This manual has been prepared to introduce the inspection standards for the housing retrofit grant, and their associated step by step procedures for construction.

This manual will be used by all the engineers who are working for the reconstruction, and have been deployed by the GoN to carry out inspections.

The manual has been divided into four parts and two annexes:

PART- A: Seismic damage and intervention PART- B: Seismic deficiencies and intervention PART- C: Ready to use seismic retrofit designs PART- D: Construction Sequences Annex 1: Typical structural drawings Annex 2: Annex 2 : EMS Damage Grade

Dr. Hari Ram Parajuli Executive member, NRA This Page is Intentionally Left Blank

Standardization Committee, NRA for Reconstruction of Earthquake Resistant Houses

Member

Dr. Hari Ram Parajuli Er. Tapendra Bahadur Khadka Er. Ishwor Chandra Marahatta Er. Prakash Thapa Dr. Jagat Kumar Shrestha Er. Raju Babu Manandhar

Invited Experts

Prof. Dr. Prem Nath Maskey Prof. Dr. Hikmat Raj Joshi Prof. Dr. Gokarna Bahadur Motra Dr. Ramesh Guragain Dr. Hiroshi Imai Dr. Narayan Marasini Er. Hima Gurubacharya Er. Kuber Bogati Er. Rajani Prajapati Er. Kirty Tiwari Er. Jyoti Mani Bhattarai Er. Mahohar Raj Bhandari Er. Purna P. Kadariya Er. Parikshit Kadariya Er. Rajkaji Shrestha Er. Manoj Nakarmi

Chairman (Executive member, NRA) Member (MoUD-CLPIU) Member (Project Director, MoFALD-CLPIU) Member (Joint-secretary, NRA) IOE,TU Member (Joint-secretary, NRA)

IOE, TU IOE, TU IOE, TU Deputy Ex. Director, NSET Consultant, JICA National Technical Co-ordinator, HRRP Director, NSET National Technical Co-ordination officer, HRRP HRRP Senior St. Er., NSET/Baliyoghar Senior St. Er., NSET/Baliyoghar St. Er. NSET/Baliyoghar Adviser, NRA, Private consulting Adviser, NRA, Ex-secretary GoN Senior Division Engineer, MoUD-CLPIU Senior Division Engineer, NRA Building code section, DUDBC

Graphics Design

Chandan Ranamagar

NSET/Baliyoghar

Drawing

Rachana Kansakar

NSET/Baliyoghar

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We appreciate Partner Organisations' (POs) work to review and contribute to the draft manual, especially Build Change, Catholic Relief Services, World vision Nepal, and so on .

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Standardization Committee, NRA for Reconstruction of Earthquake Resistant Houses

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BACKGROUND

The damage assessment carried out by the NRA and the Central Bureau of Statistics (CBS) categorised damaged houses by damage grade and technical solution. Almost 25,000 households across the 31 earthquake affected districts were categorised as damage grade 2 and major technical solution or damage grade 3 and minor technical solution. Under the Government of Nepal (GoN) housing reconstruction programme, a housing retrofit grant of 100,000 NPRs will be available to these households if their retrofit complies with the relevant standards and specifications.

This manual outlines these standards and specifications as well as the minimum intervention works required to carry out the retrofitting.

The manual discusses two levels of intervention works which are necessary for damaged buildings and that ensure a life safety level of performance under the standards set out in the Nepal National Building Code, NBC 105: 1994.

There are various methods of repair and retrofitting for earthquake damaged masonry structures in different categories. Where repair methods are applied it is expected that the structure will be restroed to its pre-earthquake condition, whereas the application of retrofitting methods is expected to increase the strength and ductility of the structure beyond its original condition. This manual incorporates repair and retrofit methods considering their suitability in terms of material availability, affordability, and ease of construction.

This manual has been developed to support the engineers responsible for the compliance inspection process. The engineers will use this manual to provide advice and guidance to households for the implementation of required repair and retrofitting strategies.

Objectives of this manual:

- To set the minimum criteria to provide the tranches under the retrofitting grant
- Cover policies for distribution of tranches with minimum technical intervention options (economical and 31 district orientated.)
- References are taken from published documents and recent researches
- To support engineers for inspection, help them to provide advice and guidance to households

Repair and retrofitting works, carried out using approved and published GoN documents, are also considered as a part of this manual.

SCOPE

Applicability

The repair and retrofitting strategies set forth in this manual are applicable only for residential houses categorised as damage grade 2 (major) or 3 (minor) after Gorkha earthquake 2015 under the GoN housing reconstruction programme. The manual intends to achieve the minimum acceptable structural safety envisioned in NBC 105: 1994 after completing two levels of intervention.

The designs mentioned in the manual are ready-to-use designs for all structural components, but some provisions mentioned are set as advisory measures.

Limitations

The repair and retrofitting strategies are only for damaged non-engineered residential buildings.

This manual has certain limitations and is only relevant for buildings which are:

- I. Residential and fall under category 'C' and 'D' of NBC.
- Category "A": Modern building to be built, based on the international state-of-the-art, also in pursuance of the building codes to be followed in developed countries.
- ✓ Category "B": Buildings with plinth area of more than One Thousand square feet, with more than three floors including the ground floor or with structural span of more than 4.5 meters.
- ✓ Category "C": Buildings with plinth area of up to One Thousand square feet, with up to three floors including the ground floor or with structural span of up to 4.5 meters.
- Category "D": Small houses, sheds made of baked or unbaked brick, stone, clay, bamboo, grass etc., except those set forth in clauses (a), (b) and (c).

* If the intervention has already been completed as per, or similar to, the strategies outlined in this manual, Government of Nepal published documents, or as per international practices, and are based on codal provision ensuring life safety with quality construction, then applications can be forwarded only after thorough engineering judgement

Few definition

Seismic Damage

According to post earthquake damage assessment carried out as per EMS 98 scale, level of damages found in masonry buildings are of grade 1 to grade 5 (refer annex 2). The structural components which helps in smooth transmit of loads in masonry building are (i) foundation, (ii) structural masonry walls, (iii) roof/floor and (iv) connections (are vital which maintains integrity of the structural systems) where as remaining components such (i) partition walls, (ii) gable walls, (iii) Chimney, (iv) false ceiling, (v) decorative components etc. are non-structural components of the building. The damages in structural and non-structural components of buildings due to earthquake are seismic damage. Repair shall be done to the non-structural components against damage.

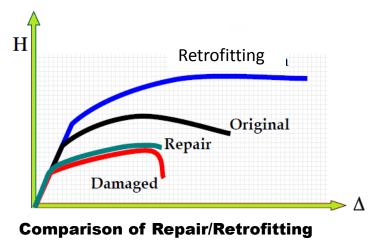
Seismic Deficiencies

Building system, configuration, lateral force resisting system are basic vulnerability factors to the seismic performance to the building in future earthquake. Retrofitting can be done to overcome seismic deficiencies of buildings. The masonry buildings which are partially damaged due to Gorkha earthquake, which comes under DG2-major and DG3-minor and are eligible to get housing grant under "repair and retrofit" category shall have life safety level of performance envisioned by NBC 105:1994 to complete the tranches.

Intervention works

Repair and retrofitting are intervention works in seismic damaged or capacity deficient structures:

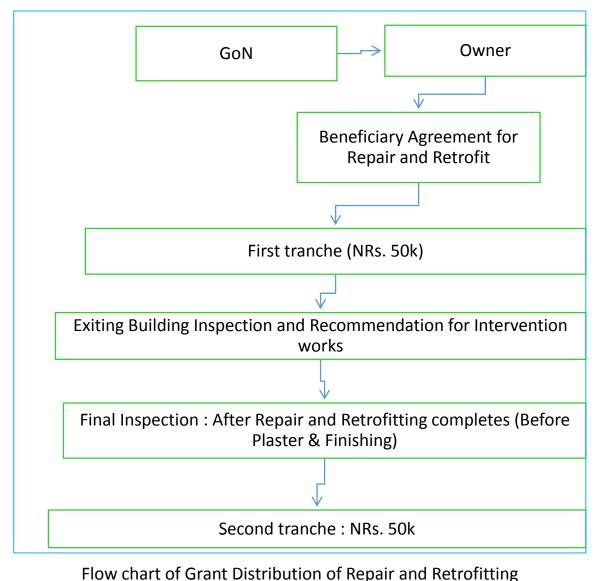
- **Repair** : Repair does not improve the structural strength of the building and very deceptive for meeting the strength requirements of the next earthquake.
- **Retrofitting**: This removes the seismic deficiencies and improves the response of existing un-reinforced masonry buildings to both gravity and seismic loads it improves the "box type" behavior and increases the flexural strength of un-reinforced walls and piers. Improvement in configuration, load path, redundancy, connections, ductility and capacity etc falls under this.



Level of Intervention

Level of Interventions required to receive the grant:

This guideline presents repair and retrofitting methodologies for stone masonry and brick masonry buildings. For retrofitting, it consists of retrofitting design and outcomes of the sample buildings taken from the earthquake effected areas. Also for those building which can be modified to match with sample buildings, modification methodologies are given. For those buildings which are not within the scope of the manual (such as span restrictions, height restrictions, number of storey etc.), this manual is not applicable however, grant will be given if detail retrofit analysis and design is carried out.



Grant distribution process:

PART-A: Seismic damage and intervention

This part deals with seismic damages and possible intervention that needs to turn the building in to pre-earthquake condition.

1. Foundation damage and mitigation work

[Key Problem]

Potential damages in foundation are as follows :

- F.1 Cracked stone masonry
- F.2 Settlement of Foundation
- F.3 Bulged Stone Walls
- F.4 Dislocations and Loose Stone

F.5 Stone foundation wall interruption, removal of portions of the wall, & loss of structural integrity

[Repair Solution]

Repair solution on corresponding damages in foundation listed above, are :

F.1 For Minor cracks : action #01

F.2 Settlement of Foundation :May be possible with grouting if it is only minor to moderate crack (Action #04), Else more soil intervention may require

F.3 Bulged Stone Walls : action #04

F.4 Dislocations and Loose Stone : Action #04

F.5 Stone foundation wall interruption, removal of portions of the wall & loss of structural integrity : Action #04

[Retrofitting Solution]

FR.1 Foundation improvements : if the existing foundation size is not sufficient then improvement in foundation is required, else repair or restore of damaged works as above is sufficient. Foundation improvement can be done as explained (Refer part B).

[Note] : Level/extent of damage can be quantify on the basis of post earthquake damage assignment knowledge and appropriate repair/retrofit solution can be recommended accordingly.

1.1 Foundation Damage and mitigation work

Problem

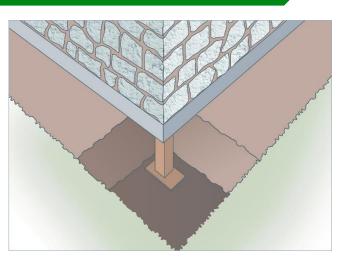


F.3 Bulged and leaning stone Foundation

Solution: Repair works

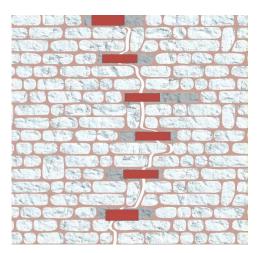


F.2Settlement of Foundation



Reconstruction of Foundation with proper safety

Note : Provide sufficient props and temporary support, reconstruct the wall footing as new. Also, increase soil bearing capacity or provide additional retaining structures if needed.



Repair Foundation

Note : Depending upon size of cracks, apply appropriate solution discussed on part B1. Also ensure the sufficiency of foundation size, if yes, then no improvement is required in foundation.

2. Roof/floor partial collapse and mitigation works

[Key Problem]

Potential damages in floor and/or roof are as follows :

- R.1 : Partial to heavy damage on gable wall
- R.2 : Sliding of roof materials (stone slate or clay tiles)
- R.3: Roof connections failure
- R.4 : Floor connection
- R.5 : Floor to wall connection
- R.6: Roof to wall connection
- R.7 :Wall to wall connection

[Repair Solution]

Repair solution on corresponding damages in floor and/or listed above, are :

R.1 : Remove heavy material replace with lighter materials

- R.2.: Replace damaged tiles and anchor all the tiles
- R.3: Provide new appropriate roof/floor connection

[Retrofit Solution]

F(RF).2 Improve Roof-to-Wall connection, Floor-to-Wall connection, improve connections in Roof/Floor (stiffening the Roof/Floor), provide bracing at Roof/Floor. Improvements are described in part B.

R1.Partial to heavy damage of Gable wall

Problem : Gable wall collapse



R1.1 Gable wall toppling

R.1.2 Gable wall toppling

Solution : Remove heavy material replace with the lighter materials



Use of C.G.I. sheet at Gable part



Use of timber planks at Gable part

R.2:Sliding of Roofing materials

Problem : Sliding of roof tiles (stone slate or clay tiles)



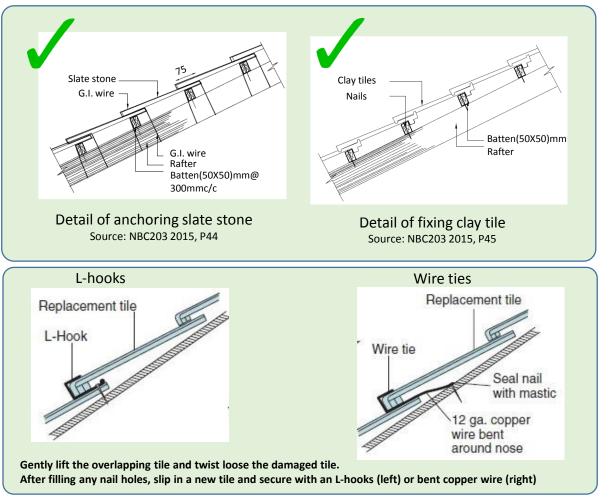
R.2.1 Slate stone roof



R.2.2 Clay tile roof

Solution

- Replace damaged tiles.
- Using appropriate correct fixing method for roofing materials.
- Connect the roof with the roof band by inserting reinforcement or GI sheet.
- Slatestone and clay tiles should be properly anchored to purlin as NBC.



R.3:Roof connection failure

Problem : Inadequate roof connections or connections failure



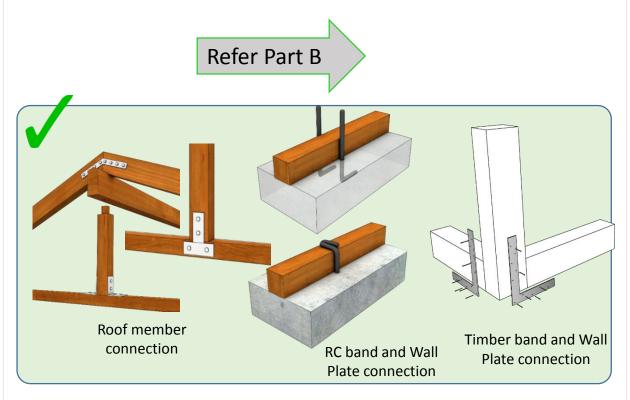
R.3.1 Purlin detached from rafter due to inadequate nailing



R.3.2 Connection of wooden truss

Solution

- Use a continuous wall plate, ridge and purlins to tie the rafters or trusses together.
- Stiffening of roof
 - Diagonal straps with steel nut bolts or metal nails
 - Diagonal steel truss with steel nut bolts or metal nails
 - Timber bracing with metal nails or timber nails



R.3:Roof connection failure

Refer Part B

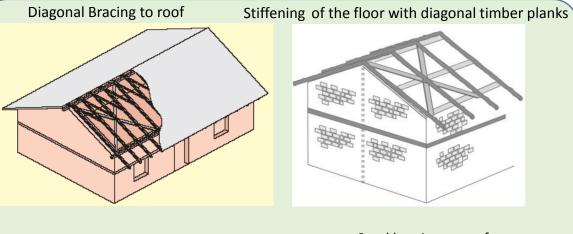
Problem :Roof/floor bracing missing



R.3.3 Bracing missing in inclined roof

Solution

- Provide X-bracing at end bays on each sloppy side
- Provide additional roof/floor member as needed



Diagonal Steel bracing to roof



Steel bracing to roof



Flexible diaphragm improvements

R.4: Floor connection

Problem : Inadequate roof connections or connections failure



R.4.1 view inside the attic showing the rafters and the absence of collar beams and joist connections to the rafters

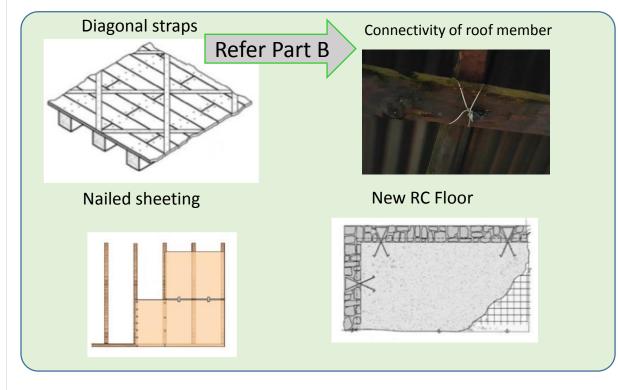


R.4.2 Inadequate and poor floor members

Solution

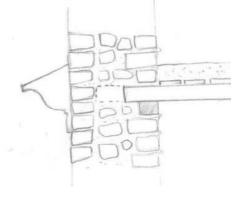
- Use a continuous wall plate, ridge and purlins to tie the rafters or trusses together.
- Stiffening of roof
 - Plywood overlay
 - Diagonal straps
 - Nailed sheeting

- Concrete overlay
- Plank overlay
- Diagonal steel truss
- Timber bracing
- New RC floor



R.5: Floor to wall connection

Problem : Inadequate roof connections or connections failure



R.5.1 Floor joists supported by half of the wall width (insufficient anchorage)

R.5.2Wall-to-floor connection *parallel to the joists (insufficient anchorage)*

Solution



Steel Strap



Timber block

Anchored with splint and bandage at floor level

R.6:Roof to wall connection

Problem : Inadequate roof connections or connections failure





R.6.1 Damaged buildings showing wall-to-floor(joist) connection in Bungamati

Solution

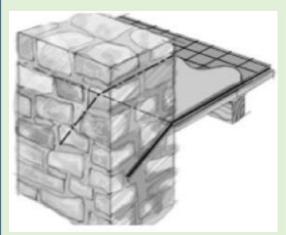
- Anchors ties
 - Anchor to joist
 - Wall anchors
 - Connector element
 - Combined methods

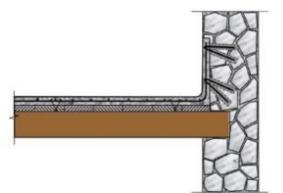
Anchor inlay into wall



Refer Part B

Anchors in wall fold overlay





R.6:Roof to wall connection

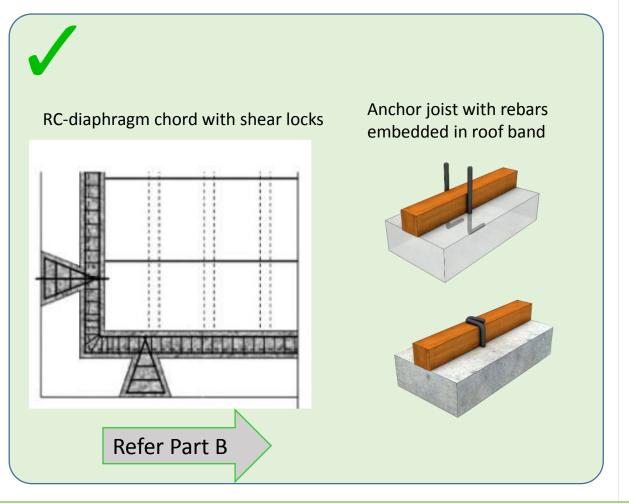
Problem : Inadequate roof connections or connections failure





Solution

Use a continuous wall plate, ridge and purlins to tie the rafters or trusses together.



3. Cracks in wall and mitigation works

Seismic damage pattern

Potential damages in structural and non-structural wall are as follows :

- C.1 : Minor cracks
- C.2 : Major cracks
- C.3 : Heavy cracks
- C.4 : Out-of-plane failure of walls
- C.5 : Wall to wall connection

Repair Solution

Repair solution on corresponding damages in wall listed above, are described in repair mitigation part D (D.1). From repair mitigation, apply appropriate options depending upon extend of damage.

Retrofit Solution

R.C.1. : For improvement in capacity of wall, retrofitting options are described in retrofitting Part-C and Part D (D.1). Relevant options can be selected and apply in the building according to building typologies

Common types of earthquake induced damages in masonry building

Problem :



Diagonal Cracks: Diagonal cracks on the walls are the result of in plane bending and shear force. When the in plane bending and shear capacity of the walls are exceeded, such diagonal X cracks are formed.



Corner Separation: The lack of proper connection between the orthogonal walls result in Corner separation



Failure of Gable wall: Lack of proper anchorage of gable wall with the roof results into failure of gable wall. This is the most common type of failure pattern in masonry buildings.

Common types of earthquake induced damages in masonry building

Problem :



Delamination:

Stone masonry walls have two exterior vertical layers (called wythes) of large stones, filled in between with loose stone rubble and mud mortar. There is no any connection between the two wythes of the wall. This causes bulging/separation of walls in the horizontal direction into two distinct wythes during earthquake. Delamination is very common in stone in mud buildings



Collapse of wall: When the out of plane bending capacity of wall is exceeded, partial or complete collapse of wall happens.



Diaphragm failure: Lack of proper connection of diaphragm: floor/ roof with the wall result in failure of floor/roof.

Also Inadequate size of column and beam

C.1:Minor Cracks

Problem : Minor In-Plane Cracks in the building







C1.1 Low strength masonry : Stone Masonry in Mud mortar

C.1.2 Low strength masonry : Brick masonry in mud mortar

C.1.3 Brick masonry in cement mortar

Definition of Damage		
Damage Grade	Damage level	Description of damage
DG1	Slight damage	Hairline cracks

Repair and Retrofitting

Repair Solution

• Pressure injection of cement grout or mixture of cement and mud : or Action #1

C.2:Moderate Cracks

Problem : Moderate In-Plane Cracks in the building



C2.1 Low strength masonry : Stone Masonry in Mud mortar



C.2.2 Low strength masonry : Brick masonry in mud mortar

C2.3 Brick masonry in cement mortar

Definition of Damage		
Damage Grade	Damage level	Description of damage
DG2	Moderate damage	Cracks up to 5 mm wide

Repair and Retrofitting

Repair Solution

- Grouting : Action #1
- Stitching

C.3 :Heavy Cracks

Problem : Heavy In-Plane Cracks in the building



C.3.1 Low strength masonry : Stone Masonry in Mud mortar



C.3.2 Low strength masonry : Brick masonry in mud mortar

C.3.3Brick masonry in cement mortar

Definition of Damage		
Damage Grade	Damage level	Description of damage
DG3	Heavy damage	Cracks greater than 5 mm wide or wall material dislodge
Repair and Retrofitting		

Repair Solution

- Through stones and Cement grouting in cracks : Action #4
- Rebuilt the portion of wall or wall cracked

C.4 :Delamination of walls

Problem : Wall-wythes separation



C.4.1 Low strength masonry : SMM



C.4.2 Low strength masonry : BMM

Solution

- Rebuilding / wall overlay
- Through wall-anchors

R.7: Wall to wall connection

Problem : Corner separation



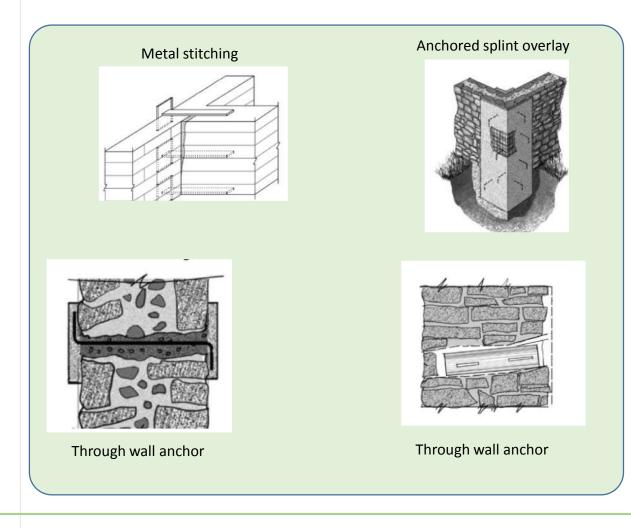
R.7.1 Low strength masonry : SMC



R.7.2Low strength masonry : SMM

Solution

• Provide additional stiches



PART-B : Seismic deficiencies and intervention

Additional intervention may required with respect to inherent structural deficiencies after turning the building in to pre earthquake condition as per part A. This part deals with possible deficiencies in the masonry buildings and possible improvement measures. Probable intervention are as follows:

- 1) Foundation improvement
- 2) Configuration and load path improvement
- 3) Connection improvement between wall to wall
- 4) Connection improvement between wall to floor
- 5) Connection improvement between wall to roof
- 6) Stiffening of floor in their plane
- 7) Stiffening of roof in their plane
- 8) Tying of parapet wall
- 9) Tying of gable wall
- 10) Capacity improvement of structural wall with splint and bandage using:

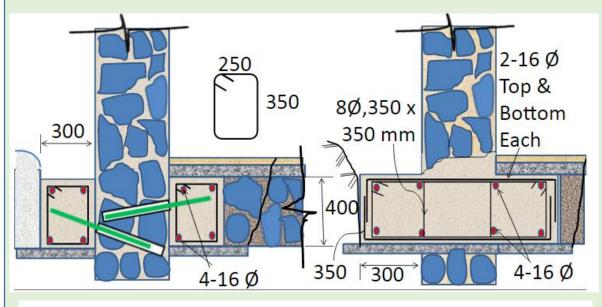
Opti on	For preventing global failure (Splint & bandage)	For present local failure control (to avoid effects due to lack of through stone etc.)	Remarks
01	Welded GI wire mesh	GI wires in remaining part	The following chapter covers brief about sketches
02	GI wire mesh	PP band in remaining part	and drawings which are informative only. For more
03	Wooden section	GI wires in remaining part	details and clarity, refer annex 1 (separate volume in A3 paper sheets).
04	RCC	GI wires in remaining part	

1. 0 Foundation improvement

General :

- Measures regarding strengthening of foundations are usually taken as part of seismic retrofit of a building. Geotechnical advice is required and specialized solutions in cases where masonry building has been damaged due to soil failure.
- In cases where no soil failure was observed foundations still may need to be strengthened when introducing new vertical structural members like tie-columns or shear walls.
- Interventions to the foundation system are also required due to deterioration of structural materials with time as well as improve the integrity of the building.
- Existing old masonry buildings are often without no or insufficient foundations. The vertical loads are transferred to the soil directly through the basement wall or foundation. In such cases construction of RC strip foundations under the basement walls can be applied. Depending on access limitations or ownership boundaries, the new strip foundations can be constructed by stitching to the sides of the existing walls or foundations.
- Before strengthening the existing foundations, the walls are first consolidated by grouting (cement or soil or stabilized soil.

To avoid disturbance to the integrity of the existing wall, during the foundation strengthening process, proper investigation & design is necessary



Refer : Schematic view of foundation strengthening with RC :

Problem



No foundation

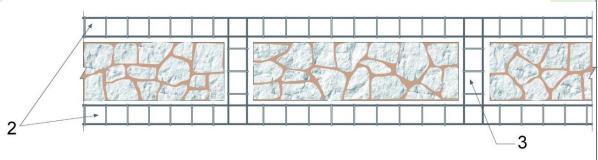


Insufficient foundation

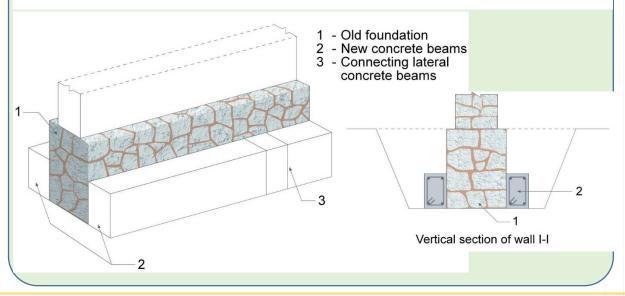
Solution

Addition of RCC foundation beam with proper connection

Schematic view of foundation strengthening with RC :



Horizontal section of wall through new RC beams



2.0 Configuration and load path improvement

Configuration :

The configuration of a building will influence the seismic performance of a building, particularly regarding the distribution of the seismic loads.

Architectural configuration : related to geometry, shape and size of building.

- The building shall be in rectangular in plan shape, ratio of maximum dimension to minimum dimension is three (3), in other case provide seismic gap.
- The projected length up to (one fifth : 1/5) of building length is acceptable.
- For small residential buildings not exceeding 100 sq.m. in plinth area with flexible floor and cross walls, the shape criterion of building can be ignored.
- The cantilever-projection of roof/floor, where provided, is acceptable but load bearing wall shall be replaced with lighter material over such cantilever-projections.
- If the load bearing wall continues from ground floor to first floor on the same vertical line, vertical setback is allowable.
- The maximum storey of building is two plus attic.

Structural configuration : related to size and location of structural members in the building

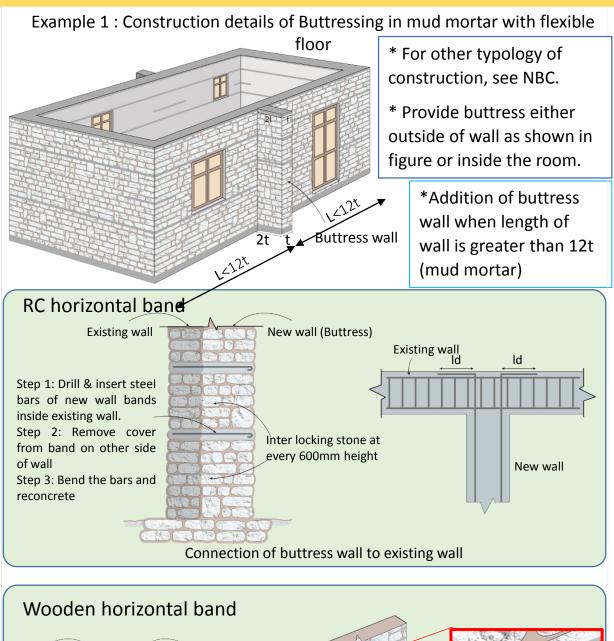
- Number of wall : there shall be two wall in each direction or equivalent system.
- Minimum wall thickness : equal or more than 230mm and 350mm for stone and brick respectively.
- Clear span of un supported wall : The span of wall up to 12 times of wall thickness is acceptable in one direction, in case where wall thickness is more than 350mm. Else, new wall or buttress walls should be constructed.
- Size of room : The size of room is restricted to 13.5 sq.m only for those houses with RCC slab. The thickness of RCC slab should be 115mm -125mm
- Height of wall : The height to wall thickness ratio of a wall shall not be more than 1:8 for stone masonry and 1:12 for brick masonry.

Load Path :

The structure shall contain at least one rational and complete load path for seismic forces from any horizontal direction so that they can transfer all inertial forces in the building to the foundation.

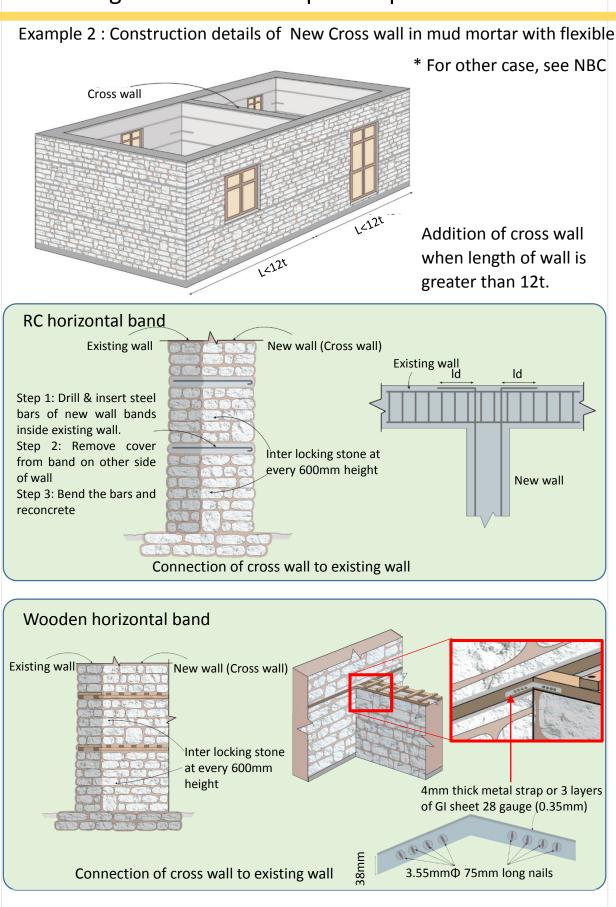
• To improve load path, size of opening can be reduced or closed with proper connection between new and old walling materials.

2.1 Configuration and load path improvement...



Existing wall New wall (Buttress) Inter locking stone at every 600mm height Mew wall (Buttress) Amm thick metal strap or 3 layers of GI sheet 28 gauge (0.35mm) 3.55mmФ 75mm long nails

2.2 Configuration and load path improvement...



Option 1: anchored splint (using RC elements with rebar or G.I. wire mesh) at corner or T-Junction in both side of wall with proper connection between new and existing walling material should be done.





G I wire mesh anchored R C splint



*See details in ready to use seismic retrofit designs, summary



3.1 Connection improvement between wall-to-wall...

Option 2: anchored splint (using wooden elements) at corner or T-Junction in both side of wall with proper connection between new and existing walling material should be done.

Installation of wooden vertical member from inside of the wall





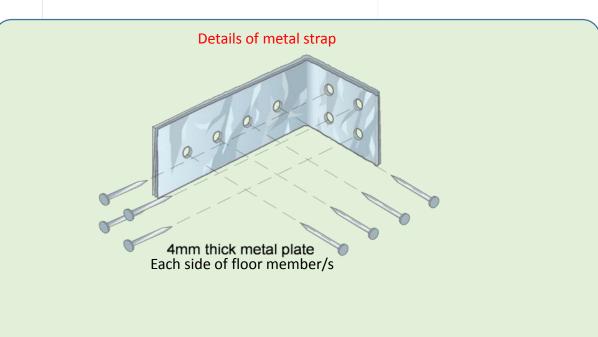
*See details in ready to use seismic retrofit designs, summary

4.0 Connection improvement between wall-to- floor

Option 1: Metal Strap :

When it is not possible to transport concrete and steel to the site:



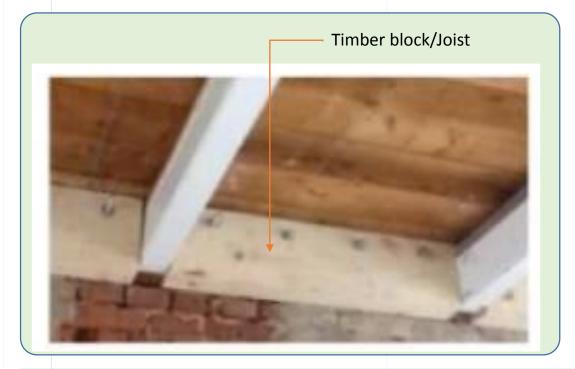


Note : Minimum four numbers of 50 mm long nails (Fe250) with Floor member and Minimum four numbers of M16 grade expansion bolts with walling material

4.1 Connection improvement between wall-to-floor...

Option 2: Timber block :

When it is not possible to transport concrete and steel to the site:



Timber block/Joist : Depth x width = 50 mmx30 mm (Hard wood) with required length and 75 mmx45 mm (Other wood) with required length

Details of timber block

5.0 Connection improvement between wall - to - roof

Option 1: Metal Strap at the top of wall:

Note : This is not required separately if retrofitting is done simultaneously. Splint is connected to the floor joist and roof rafters.



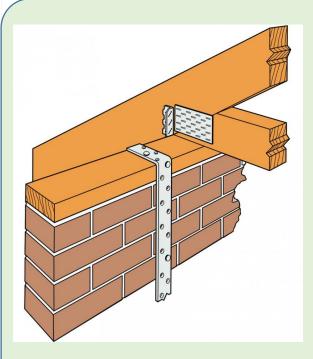
3 mm thick Height and depth : 45mmx45mm, M12 of 4 number, M16 of one number in each face @ 3.00 mat centres

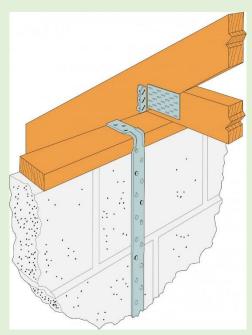
Details of Anchor plate

5.1 Connection improvement between wall-to-roof...

Option 2: Metal Strap at side of wall:

Note : This is not required separately if retrofitting is done simultaneously. Splint is connected to the floor joist and roof rafters.



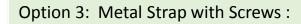


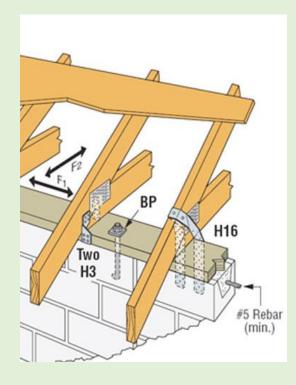


3 mm thick width and depth : 115mmx300mm, M8 of 4 number, (width side) and M8 of 8 numbers depth side face @ 4.5 m at centres

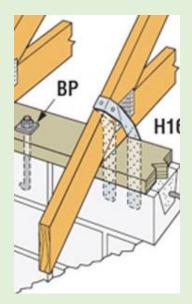
Details of Anchor plate

5.2 Connection improvement between wall-to-roof...





Note : 3 mmm thick metal strap, Minimum four numbers of 50 mm long nails (Fe250) with Floor member and Minimum four numbers of M16 grade expansion bolts with walling material

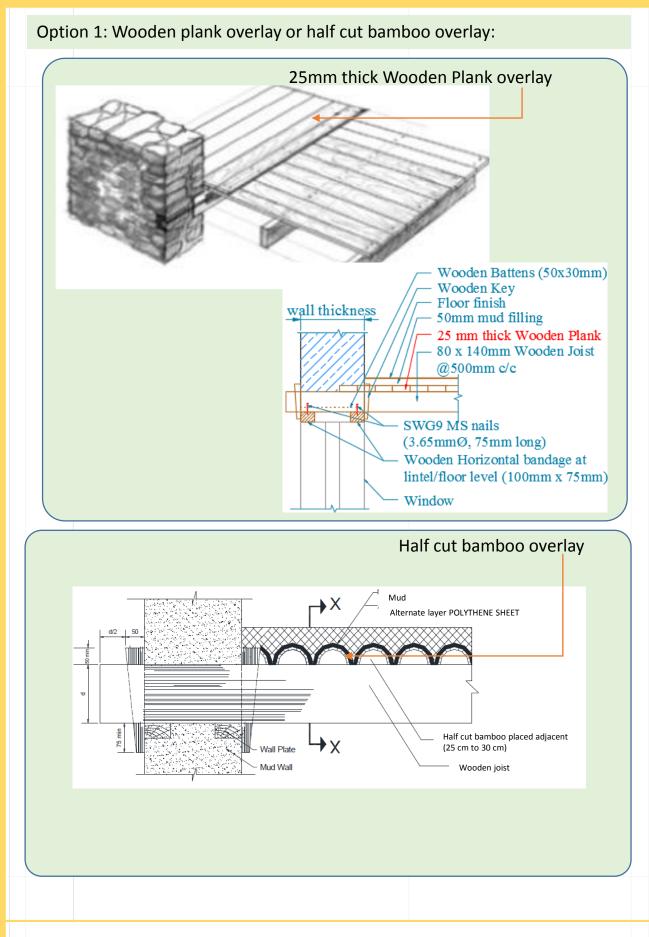


Details of Anchor plate

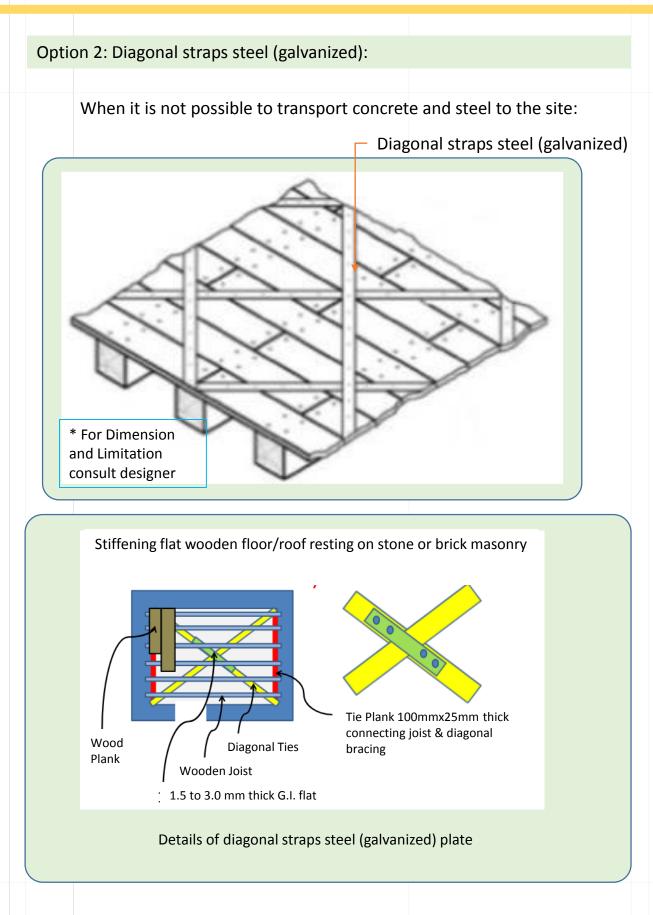
5.3 Connection improvement between wall-to-roof...

50mm wallplate 100mm screw (new) 50mm plug 100mm screw (new) 50mm plug 00mm screw (new) 50mm plug	Optic	on 4: Use of metal screw :	
Some the second	opere		
Some the set of the se			
Note : Minimum four numbers of 50 mm long nails (Fe250) with Floor member and		50mm	wallplate
		100mm screw (hex)	10mm mortar bed

6.0 Stiffening of floors in their plane

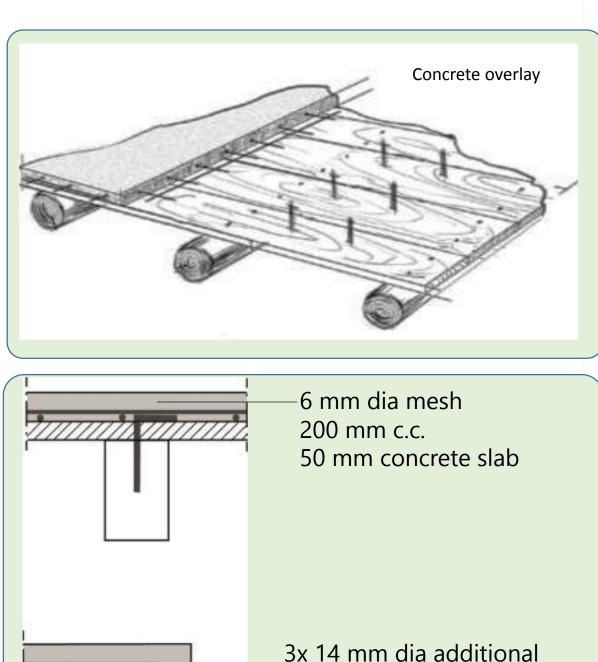


6.1 Stiffening of floors in their plane...



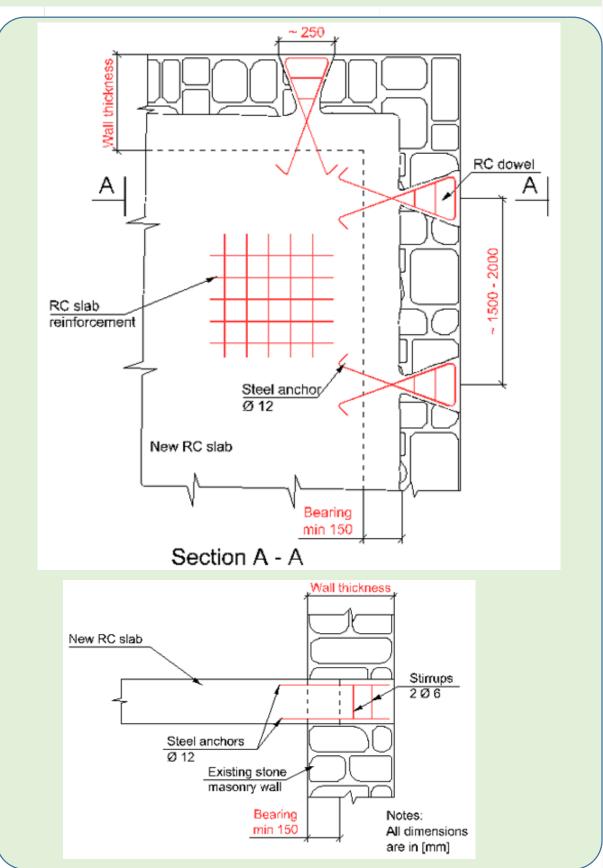
6.2 Stiffening of floors in their plane ...

Option 3: Concrete overlay:

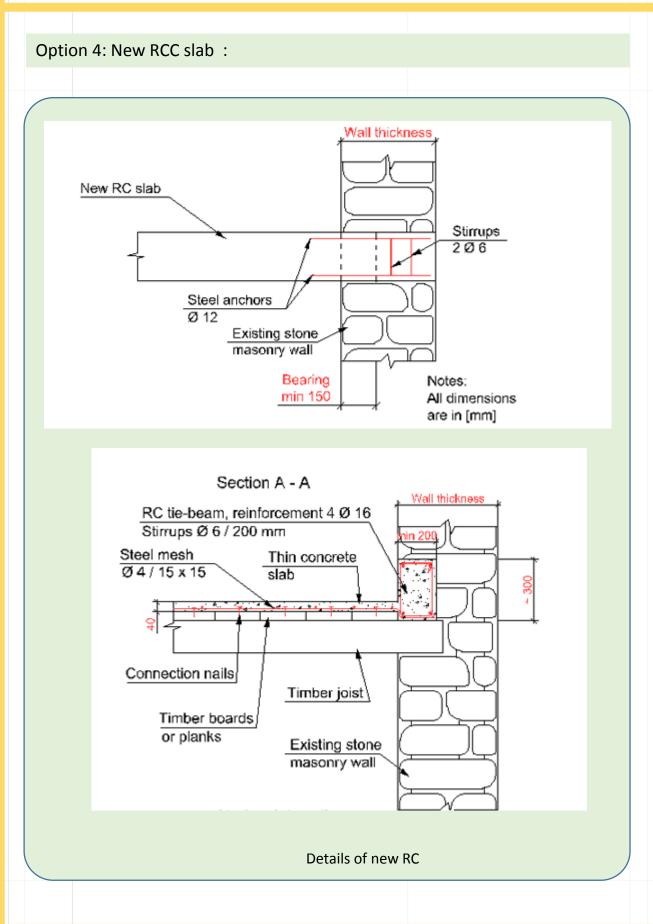


6.3 Stiffening of floors in their plane...

Option 4: New RCC slab :

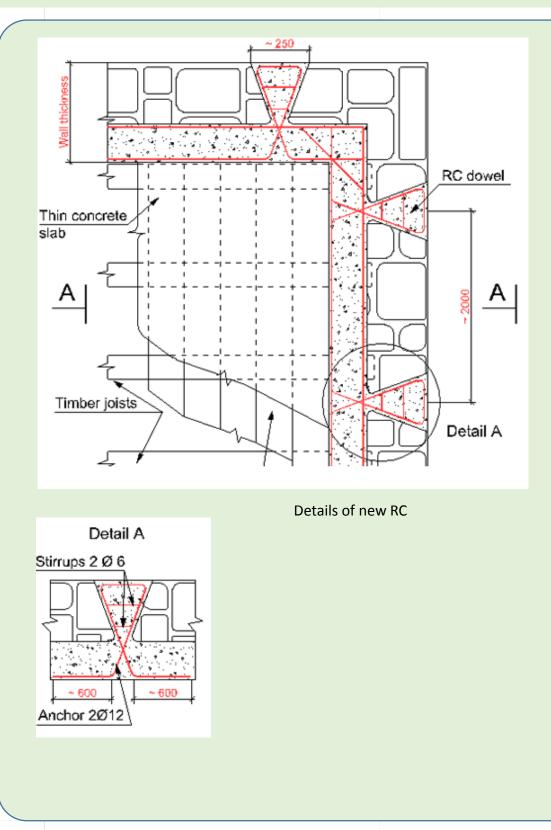


6.3 Stiffening of floors in their plane...

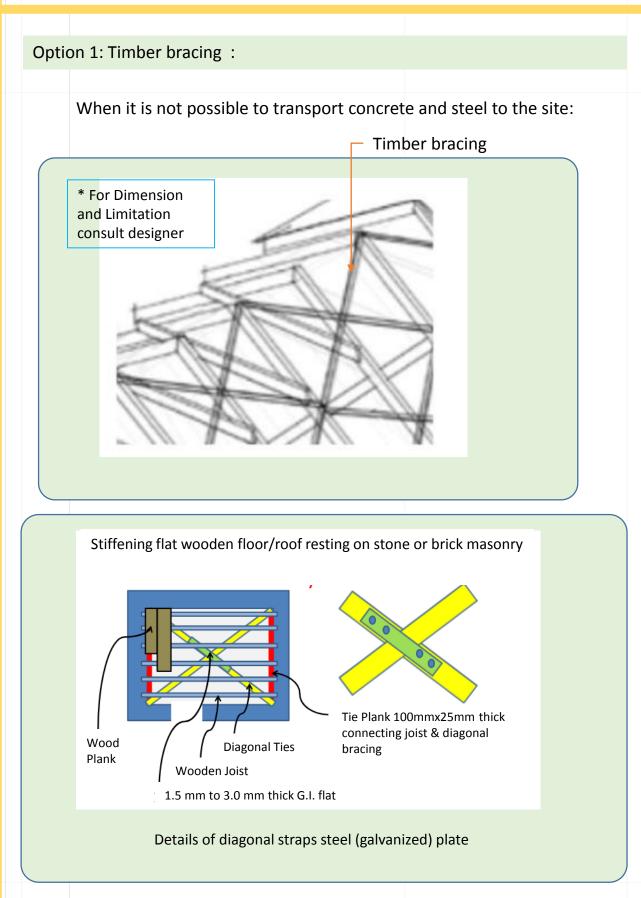


6.3 Stiffening of floors in their plane...

Option 4: New RCC slab :

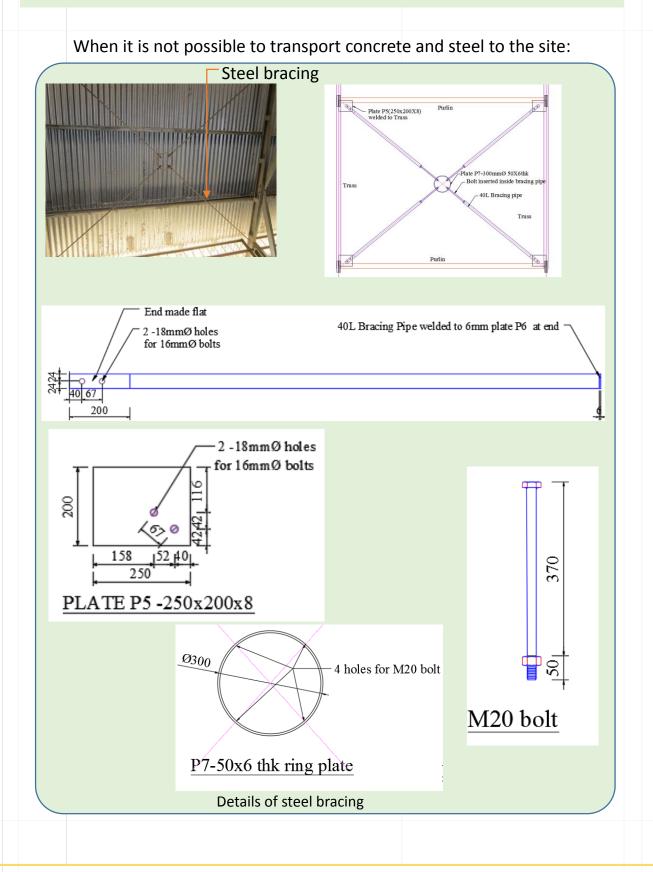


7.0 Stiffening of roofs in their plane

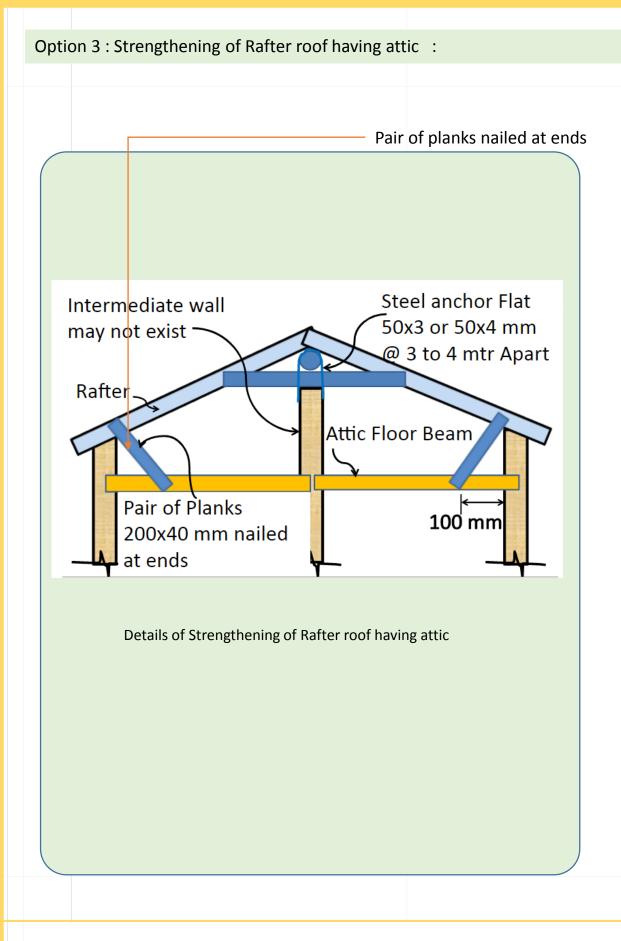


7.0 Stiffening of roofs in their plane ...

Option 2 : Steel bracing :

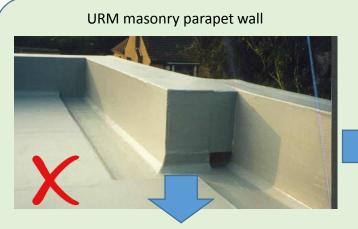


7.0 Stiffening of roofs in their plane ...



8.0 Tying of parapet walls

- Replace heavy masonry with lighter material such as steel, wooden or bamboo elements with proper connection Or
- Provide bracing or parapet band with proper connection with existing walling/floor/roof material.



Parapet wall with RC band & vertical RC post as per NBC practices



Masonry parapet braced with steel section with proper connection with slab or beam



Steel parapet with proper connection with slab or beam





Wooden parapet with proper connection with floor joists

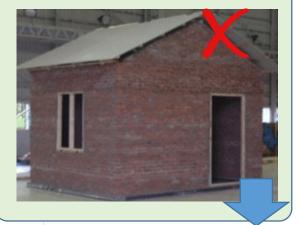




9.0 Tying of gable walls...



URM gable wall



Replace with Light weight material (CGI sheet or wooden plank)

Intervention

Replace the gable walls by lightweight materials such as galvanized iron sheets or wood panels. Where it cannot be done, confine the wall materials properly by complete jacketing or Bandages at roof and eaves level and Splint with span not exceeding 2.0m.

Provide external splint & bandage



10.0 Capacity improvement of structural wall

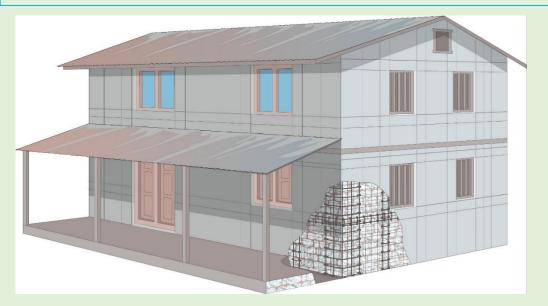
Note : This page give informative knowledge regarding the given retrofitting techniques. For more details and clarity, refer annex 1 (separate volume in A3 paper sheets).

Option 1: Retrofitting using RC splint-bandage (For preventing global failure) and GI wires in remaining part (for Local failure)



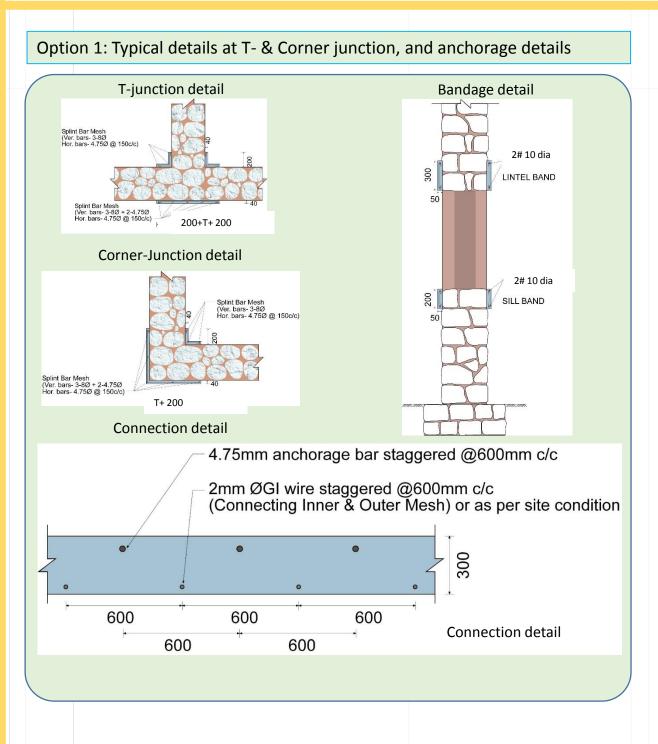
Retrofitting using rebar with concrete Splint-bandage and GI wire mesh Jacketing (Brick Masonry)

Option 1: Retrofitting using RC splint-bandage (For preventing global failure) and GI wires in remaining part (for Local failure)



Retrofitting using rebar with concrete Splint-bandage and GI wire mesh Jacketing (Stone Masonry)

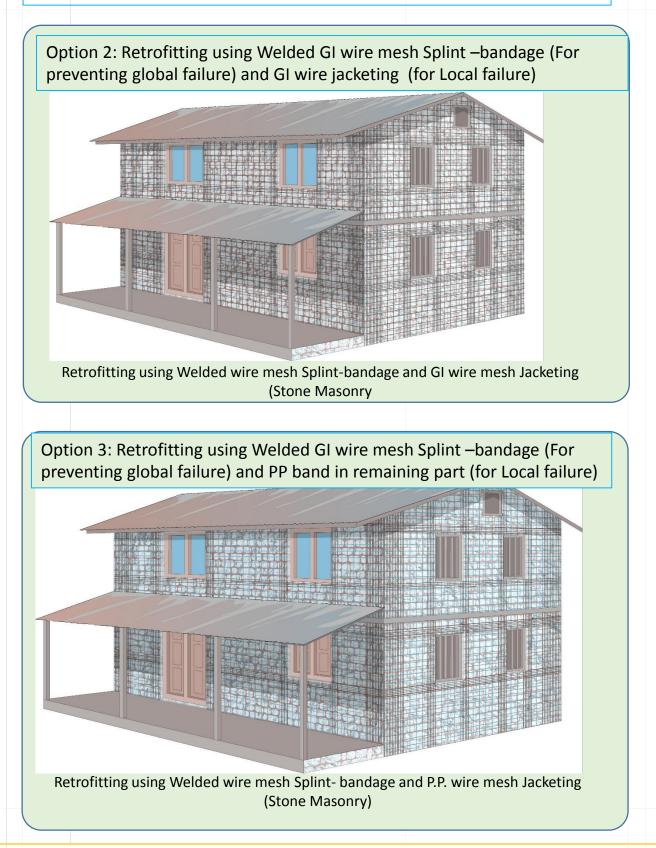
10.1 Capacity improvement of structural wall...



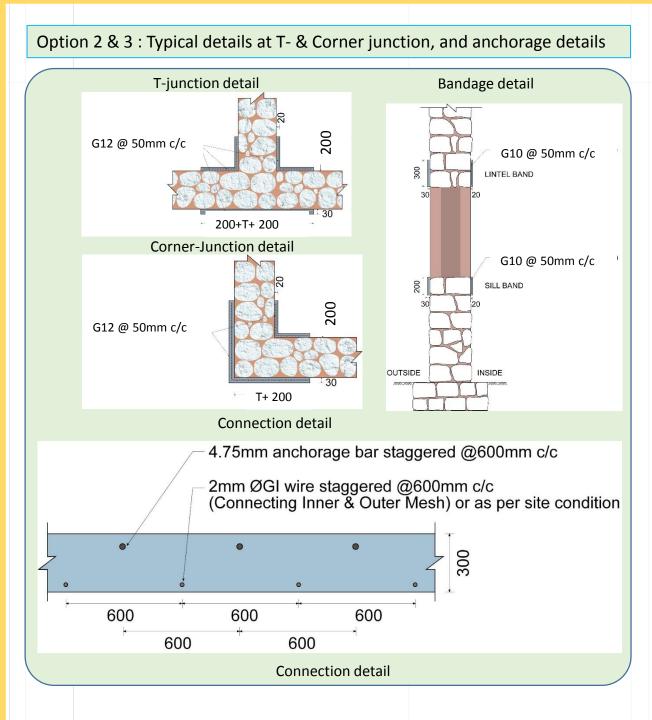
Note : This page give informative knowledge regarding the given retrofitting techniques. For more details and clarity, refer annex 1 (separate volume in A3 paper sheets).

10.2 Capacity improvement of structural wall...

Note : This page give informative knowledge regarding the given retrofitting techniques. For more details and clarity, refer annex 1 (separate volume in A3 paper sheets).



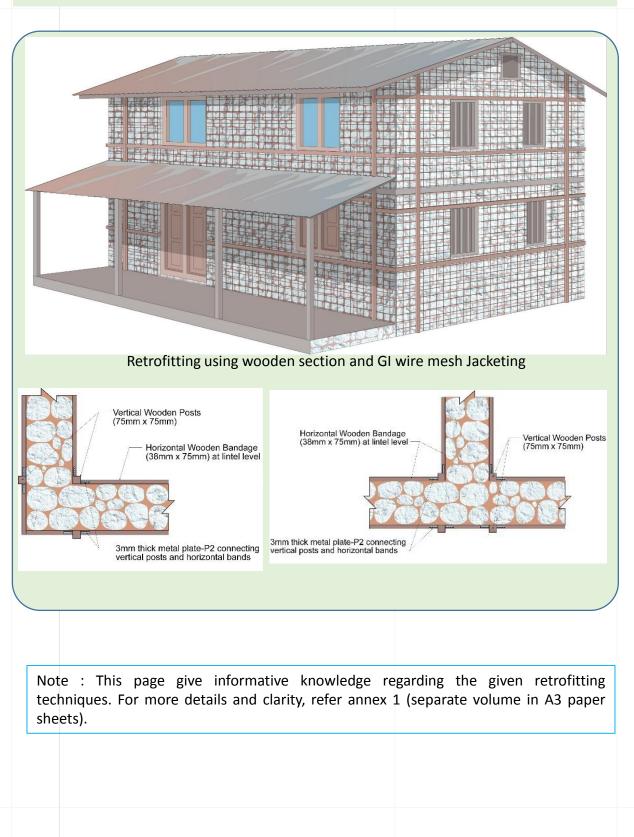
10.3 Capacity improvement of structural wall...



Note : This page give informative knowledge regarding the given retrofitting techniques. For more details and clarity, refer annex 1 (separate volume in A3 paper sheets).

10.4 Capacity improvement of structural wall...

Option 4: Retrofitting using wooden splint-bandage (For preventing global failure) and GI wires in remaining part (for Local failure)



PART-C : Ready to use seismic retrofit designs

This section presents summary of retrofit designs which are applicable in following cases :

- Retrofitting Stone masonry building
 - 1) Retrofitting stone masonry building in mud (RSMM)
 - 2) Retrofitting dry stone masonry building (RDSM)
- Retrofitting brick masonry building
 - 1) Retrofitting brick masonry building in mud with flexible floor (RBMM)
 - 2) Retrofitting brick masonry building in cement with flexible floor (RBMC1)
 - 3) Retrofitting brick masonry building in cement with rigid floor (RBMC2)

[Typical description of building]

- Number of storey : 2 plus attic (maximum), except RBMC2 which is three storey
- Storey height : 3.00 m (maximum)
- Total height: 7.0m (2 plus attic) and 9.0 m (three storey)
- Unsupported wall length: 5.40 m (maximum)
- Plinth area : 100.00 sq.m.
- Configuration and load path : is similar as mention in part B : Seismic deficiency and intervention)
- Redundant : Yes
- Material condition : Good or replaced with new material in case of damaged.

[Note:] : the design given in this section are applicable to those building which meets the description mentioned above under typical description of buildings. In other case, structural design shall be done.

Capacity improvement of structural wall : Design Summary

Option 1 : Summary of design : For RC splint & bandage

Table 3: Summary of retrofit design (applicable to RSMM, RDSM, RBMM RBMC1 : 2 plus attic storey & RBMC-2* : three storey)

S.N.	Length or Wall	Rebars Reinforcement in seismic belts with overlapping of Ld mm		
	In meter	Concrete Size (mm)	Rebar (No & diameter)	
1.	<u><</u> 5.40	300 x 40	2#10 👁	

Note : Material grade : M20 and Fe 500 or 415 , ties 4.75 mm diameter bars @ 150 m spacing.

Split : Rebar in RC seismic splint with overlapping of Ld mm,									
SN	No. of storey		Concrete size (Width x thickness	At T-Junction		At Corner Junction		At near opening	
	Concrete Grade M 20, Rebar Grade Fe 500		No	Bar 👁 (mm)	No	Bar 👁 (mm)	No	Bar 👁 (mm)	
1	One		200x40	3	8	3	8	2	8
2.	One	attic	200x40	3	8	3	8	2	8
	plus attic	Ground	200x40	3	8	3	8	2	8
3.	Two	First	200x40	3	8(10)	3	8	2	8
Gr		Ground	200x40	3	8 (10)	3	8	2	8
4.	Two plus	Attic/S econd	200x40	3	10	3	8	2	8
	attic/s econd	First	200x40	3	10	3	8	2	8
		Ground	200x40	3	10	3	8	2	8

Note :1) Material grade : M20 and Fe 500 or 415 , ties 4.75 mm diameter bars @ 150 mm spacing.

: 2) The splints which is provided to out side of the room shall be of 200+2* wall thickness and 200+ wall thickness at T –junction and at corner respectively with 2 Numbers of 4.75 diameter vertical bars additionally.

Provide G.I. wire mesh at 100 mm at cetres in horizontally and vertically to prevent local failures.

: values in parenthesis is RBMC -2

Capacity improvement of structural wall : Design Summary

Option 2 & 3 : Summary of design : For Welded G.I. Mesh splint & bandage

Table 1: Summary of retrofit design (applicable to RSMM, RDSM and RBMM)								
S.N. Length or Wall G.I. Mesh Reinforcement in seismic belts with overlapping of 300mm								
In meter Gauge No. Wide								
1.	<u><</u> 5.40	10	12 (5)	300 (200)				

Gauge, G10 = 3.25mm.

• Value in parenthesis is for sill band, remaining in lintel band

• concrete thickness 20mm & 30mm for inner and outer section.

Split : G.I. Mesh Reinforcement in seismic splint with overlapping of 300mm,(applicable to RSMM, RDSM and RBMM)										
SN	No. of storey	Storey	G	At T-Junction		At Corner Junction		At near opening		
				No	W	No	W	No	Wide	
1	One		12	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
2.	One plus attic	plus	attic	12	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)
		Ground	12	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
3.	Two	First	10	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
		Ground	10	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
4.	Two plus	attic	10	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
	attic	First	10	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	
		Ground	10	18 (5)	2X200 +T (200)	14 (5)	200 +T (200)	5 (5)	200 (200)	

Values in parenthesis () is for splint in side of the room and remaining value is for splint outside the room.

Gauge, G10 = 3.25mm, G12 = 2.64mm

Note : provide PP band or G.I. wires (G12) at 100 mm at centers vertically and horizontally to prevent local failures in remaining portion of the walls.

• concrete thickness 20mm & 30mm for inner and outer section.

Capacity improvement of structural wall : Design Summary

Option 4 : Summary of design : For wooden splint & bandage

Table 2: Summary of retrofit design	(applicable to RSMM and RDSM)

S.N.	Length or Wall	Wooden member in seismic belts with proper overlapping.		
	In meter	size	No.	Wide
1.	<u><</u> 5.40	38mmX75mm	2 (each face)	

Note : Connection using MS plate.

Split : wooden member in seismic splint with proper overlapping .						
SN	No. of storey	Storey	Size	At T- Junctio n	At Corner Junction	At near openi ng
				No	No	No
1	One		75x75	4	3	4
2.	2. One plus attic	attic	75x75	4	3	4
		Ground	75x75	4	3	4
3.	Тwo	First	75x75	4	3	4
		Ground	75x75	4	3	4
4.	Two plus attic	attic	75x75	4	3	4
		First	75x75	4	3	4
		Ground	75x75	4	3	4

Note : Connection using MS plate. Provide G.I. wire mesh of 12 gauge @100 mm at cetres in horizontally and vertically to prevent local failures.

PART-D: Construction Sequences

[This section deals with construction sequences regarding repairs measures and retrofitting measures.]

PART-D.1: Repair Process

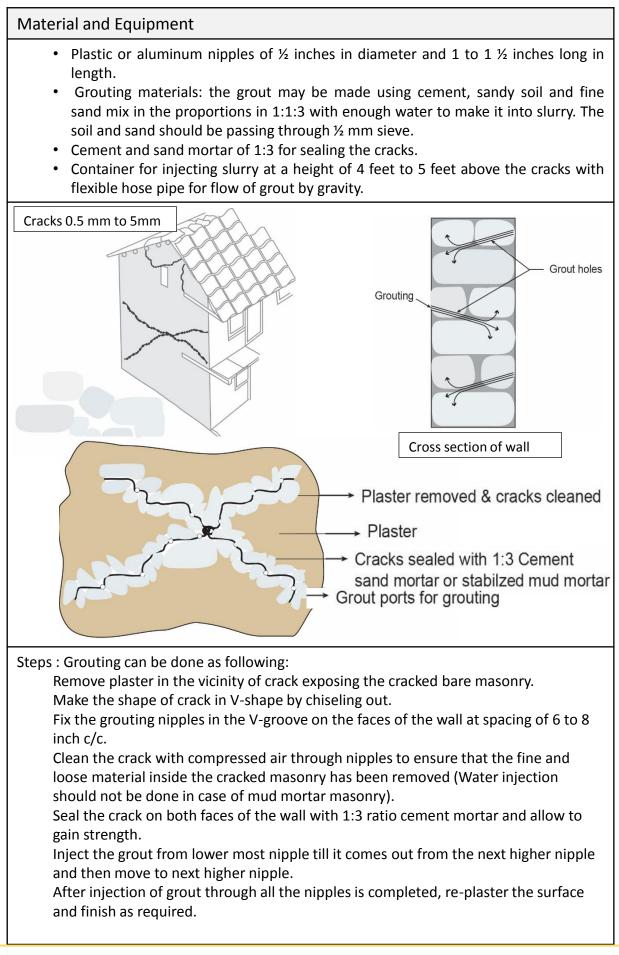
Repair does not improve the structural strength of the building and very deceptive for meeting the strength requirements of the next earthquake. Repair measures presented are :

- a) Action # 1: repair minor to cracks using grouting
- b) Action # 2: repair major cracks by fixing wire mesh
- c) Action # 3 : repair major cracks by using stitching elements
- d) Action #4: repair of damaged wall by rebuilding

PART-D.2: General Retrofitting Process

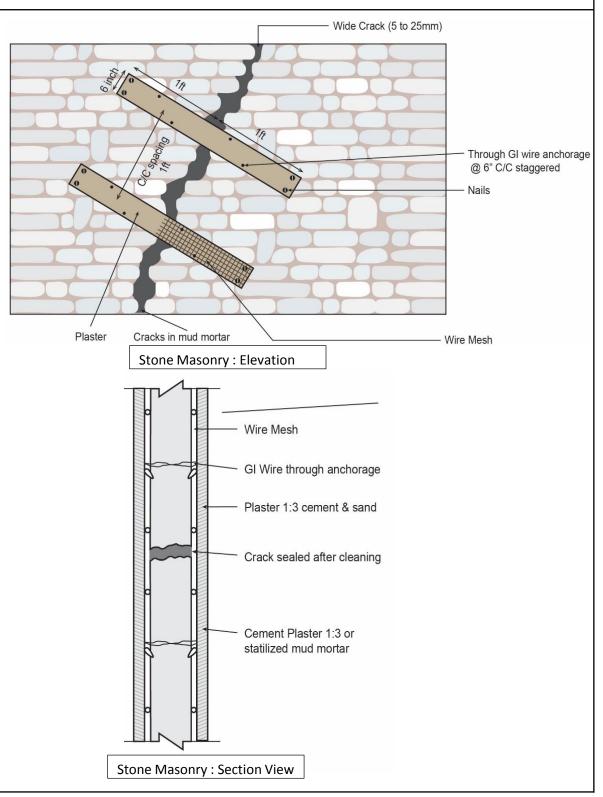
[Retrofitting measures are meant for increasing the capacity of structural components of the building. This sub section gives typical construction process of retrofitting]

Action # 1: Repair Minor To Cracks Using Grouting





- 1:3 cement-sand mortar for sealing of cracks and plastering.
- Galvanized steel wire mesh (with wires of 16 to 14 gauge i.e. 1.5mm to 2.03 mm diameter) 1 inch x 1 inch mesh size.
- Galvanized steel wire of 12 gauge i.e 3.15 mm diameter, 4 inch long nails.

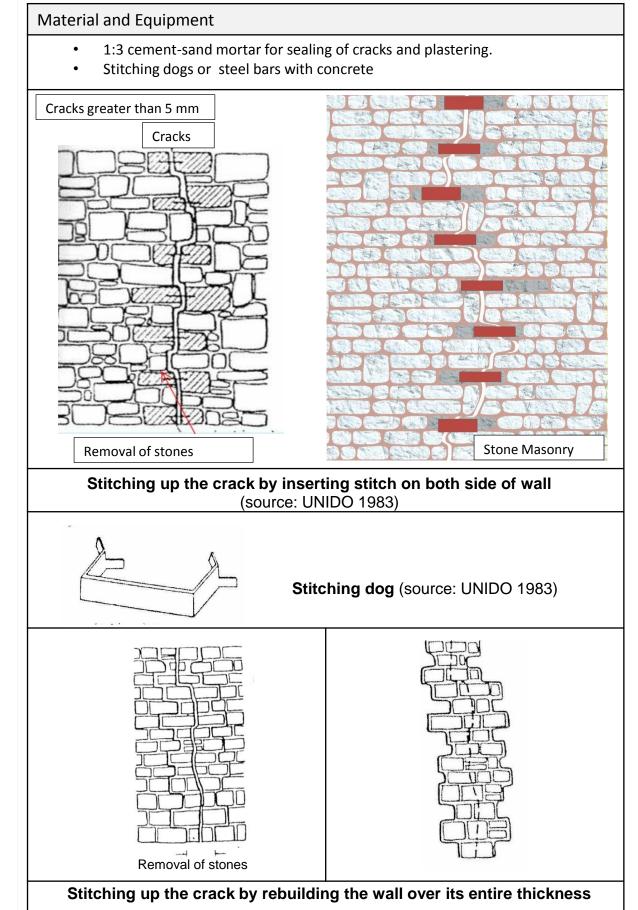


Action # 2: Repair Major Cracks By Fixing Wire Mesh...

Construction steps:

Steps : Major Cracks (crack width greater than 5 mm) can be repaired by wire mesh as following .

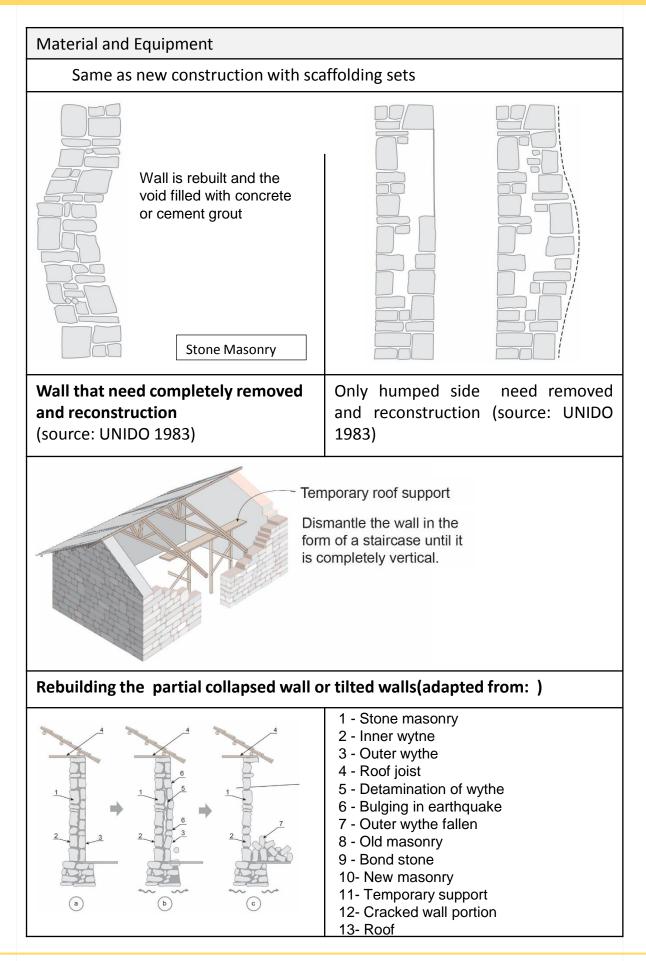
- Remove the plaster in the vicinity of crack exposing the cracked bare masonry upto around 6 inch width at both sides of the crack.
- Make the shape of crack in the V-shape by chiseling out.
- Make the throughout hole on wall by drilling on planned area of wire mesh at spacing of 6" c/c staggered.
- Clean the crack with compressed air.
- Fill the crack with 1: 3 ratio cement mortar with necessary water from both sides as deep as feasible.
- Provide wire mesh on both the faces of wall to a minimum width of 6 inch on each side of the crack and in the entire length of the crack. if it is not possible to provide in the entire length, then provide pieces of wire mesh (width not less than 6 inch) at spacing of about 1 ft.
- Clamp the mesh with the wall using nails.
- Connect the both side of mesh by galvanized wire through the throughout holes of wall.
- Plaster the meshed area with cement sand mortar of 1:3 with minimum of 1/2 inch thickness.



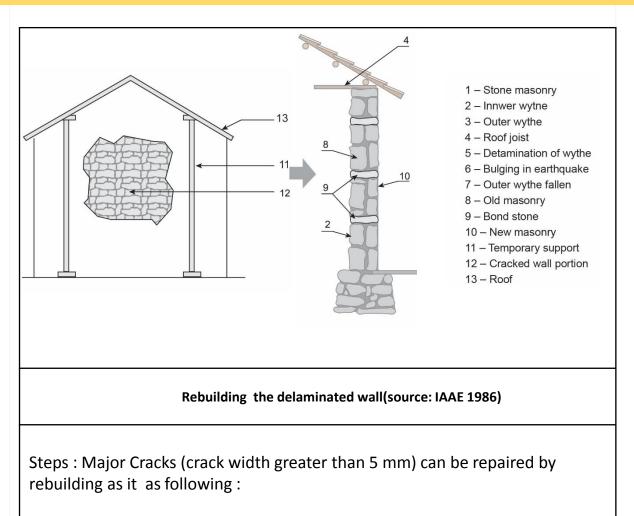
(source: UNIDO 1983)

Steps : Major Cracks (crack width greater than 5 mm) can be repaired by stitching as following :

- Remove the plaster in the vicinity of crack exposing the cracked bare masonry upto around 6 inch width at both sides of the crack.
- Make the shape of crack in the V-shape by chiseling out.
- Remove the alternate loose stones adjacent to the cracks.
- Clean the crack with compressed air.
- Add stitching dogs or steel bars with concrete on removed loose stones.
- Alternatively, stones can be removed from a zone about 6" to 9" along a vertical cracks and the wall can be reconstructed using elongated stones.
- Fill the crack with 1: 3 ratio cement mortar with necessary water from both sides as deep as feasible.
- Plaster the exposed area with cement sand mortar of 1:3 with minimum of 1/2 inch thickness.



Action #4: Repair Of Damaged Wall By Rebuilding..



- Extensive damage may occur to stone walls which require a portion of the wall to be removed and reconstructed.
- In such case, it is important to promptly install temporary shoring to support the floors and walls above that depends on the extent of damage of wall.
- When portions of the walls have permanent lateral distortion or humping throughout the width of the wall, the distressed portion of the wall must be completely removed and reconstructed.
- If the wall has a spread or humped on only one face, complete reconstruction can be avoided if the vertical face is stable enough to be used as formwork after the humped side has been taken down. Header are placed in the rebuilt wall using concrete or cement grout to completely fill all voids.
- The new portion of the wall should be constructed using the same mortar as that used in the existing construction.

• Following gives over view of construction steps, activities and description of retrofitting process.

Steps	Activities	Description		
1	Remove plaster	Plaster removed from walls in the area only where splir and bandage is to be added, dust cleaned, any hole patched up for consistent wall surface. Mud mortar rake to 25mm in case of mud mortar and 12 mm to 15 mm i case of cement plaster.		
2	Clean, repair and prepare walls	Cracks cleaned with water and grouted, thin cemen slurry for cement based construction or mud (mud cement mix) for mud based construction, applied ove the area of wall where splint and bandage is to be added with water bottle.		
3	Excavate tie beam	Excavation as per drawing, soling installed to correct level. Foundation wall brushed and washed with water to remove all mud.		
4	Tie beam ties installed	Holes drilled or cavities located as per required spacing reinforcement installed as per diagram to length of half o wall width plus 50mm (2").		
5	Tie beam ties grouted	Grout all reinforcement ties using 1:3 mortar mix, firm i place.		
6	Install splint and bandage	GI wire ties placed as per required spacing. Wire mest rolled horizontally, with corners of room at the middle of the panel. Wire mesh secured in place with tightener loops of GI wire ties, wire mesh panels bound together using binding wire with 50mm (2") overlap.		
7	Demolish walls and replace frames	Replace any damaged timber frames and install new lintel.		
8	Reconstruct wall, truss mounts, bracing mounts, install jacketing			
9	Truss mounts	Fill 300mm x 300mm (1'x1') gaps in wall with stone /concrete, cast GI wire ties and fold wire mesh on bot sides of wall across the top of wall. Cast 4x 10mm bar 600mm (2') long. 2 bars on each side of where trus beam will be located, 150mm (6") apart or as require depending on timber width.		

10	Secure wire mesh,	Wire mesh installed tight against stone wall, wire tie		
10	install reinforcement	loops tightened and tied, corners of wire mesh		
		flattened, and tie beam anchors tight, 12mm dia rod in		
		place with lap lengths 500 mm. Install vertical		
		reinforcement at splint location midway along external		
		end walls as per drawing. Formwork constructed for tie		
11	Concrete tie beams	beam, tight with no leaks. Concrete tie beams with 1:1.5:3 concrete mix. Ensure		
		good compaction using rod. When hardening fill hole		
		with water or cover with wet jute immediately, cover		
		with material or tarpaulin for shade to prevent		
12		evaporation.		
12	Plastering internal	Plastering to inner walls using 1:3 mortar mix. Plaster		
		applied in two layers. Ensure depth and good bond with		
		wall. To depth 30mm or 20mm where specified in		
		drawings. Smooth finish. Stop near top of wall. Upon		
		completion cover with wet jute material and tarpaulin		
		to stop evaporation. Apply water to jute several times a		
		day or as needed to maintain wetness.		
13	Plastering external	Plastering to outer walls using 1:3 mortar mix. Plaster		
		applied in two layers. Ensure depth and good bond with		
		wall. To depth 30mm as specified in drawings, and		
		50mm thickness at splint location as per drawing.		
		Smooth finish. Stop near top of wall. Upon completion		
		cover with wet jute material and tarpaulin to stop		
		evaporation. Apply water to jute several times a day or		
		as needed to maintain wetness.		
14	Plaster top of wall	Plaster top of wall to minimum 50mm thickness using		
		1:3 mortar mix or micro concrete 1:1.5:3 mix as advised		
		by technical supervisor. Upon completion cover with		
		wet jute material and tarpaulin to stop evaporation.		
		Apply water to jute several times a day or as needed to		
		maintain wetness.		
15	Roof installed	Roof truss timber cut, fabricated, placed and secured.		
		Truss constructed as per drawing.		
16	Install bracing	Install bracing.		

Annex 1: Typical structural drawings

[Annex 1 is compiled in A3 sheets, separate volume which is detail retrofitting drawings for type design of particular buildings. Further structural drawing can be prepared for ready to use designs presented in Part : C with study of these drawings. The drawing are of following building typologies:

- Retrofitting Stone masonry building
 - 1) Retrofitting stone masonry building in mud (RSMM)
 - 2) Retrofitting dry stone masonry building (RDSM)
- Retrofitting brick masonry building
 - 1) Retrofitting brick masonry building in mud with flexible floor (RBMM)
 - 2) Retrofitting brick masonry building in cement with flexible floor (RBMC1)
 - 3) Retrofitting brick masonry building in cement with rigid floor (RBMC2)

[Note:] : The preparation of detail drawing shall not be mandatory where ready to use design are applicable. In case of detail design submitted, only typical drawing shall be prepared.

Annex 2 : EMS Damage Grade

Classification of damage to masonry buildings



Grade 1: Negligible to slight damage



Grade 2: Moderate damage



Grade 3: Substantial to heavy damage



Grade 4: Very heavy damage



Grade 5: Destruction

Structural damage : No Non-structural damage: Slight

- Hair-line cracks in very few walls.
- Fall of small pieces of plaster only.
- Fall of loose stones from upper parts of buildings in very few cases.

Structural damage : Slight Non-structural damage: Moderate

- Cracks in many walls.
- Fall of fairly large pieces of plaster.
- Partial collapse of chimneys.

Structural damage: Moderate Non-structural damage: Heavy

- Large and extensive cracks in most walls.
- Roof tiles detach.
- Chimneys fracture at the roof line; failure of individual non-structural elements (partitions, gable walls).

Structural damage: Heavy Non-structural damage: Very heavy

Serious failure of walls; partial structural failure of roofs and floors.

Structural damage: very heavy

Total or near total collapse.

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नेपाल सरकार राष्ट्रिय पुनर्निर्माण प्राधिकरण आवास पुनर्निर्माण कार्यऋम सिंहदरवार, काठमाडौँ फोन नं.: ०१४२००२६६, ४२१११०३ इमेलः info@nra.gov.np

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON THREE STOREY BRICK MASONRY BUILDING IN CEMENT USING RC BARS

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF THREE STOREY LOW STRENGTH BRICK MASONRY BUILDING IN CEMENT USING RC BARS

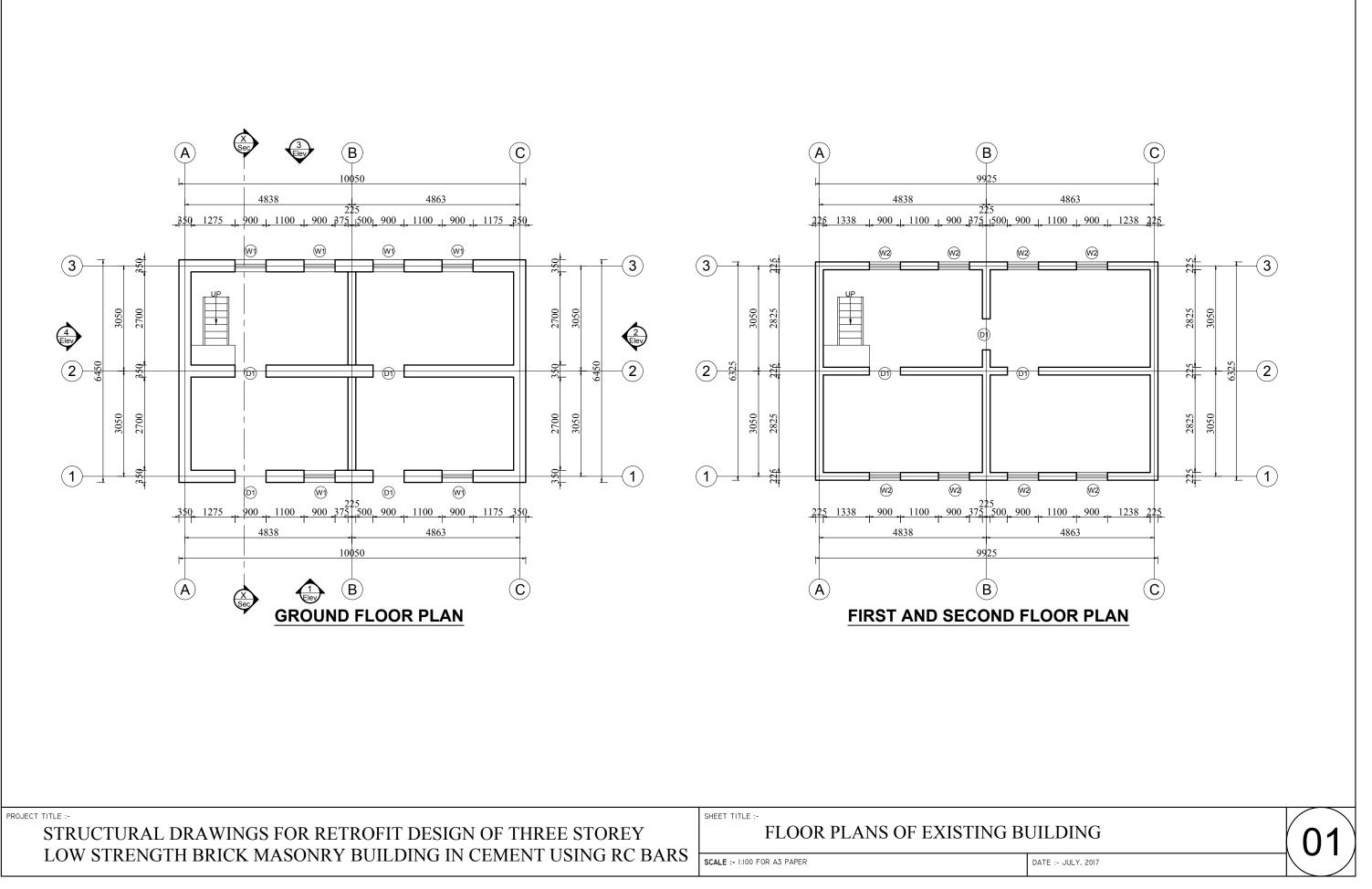
SHEET TITLE :-

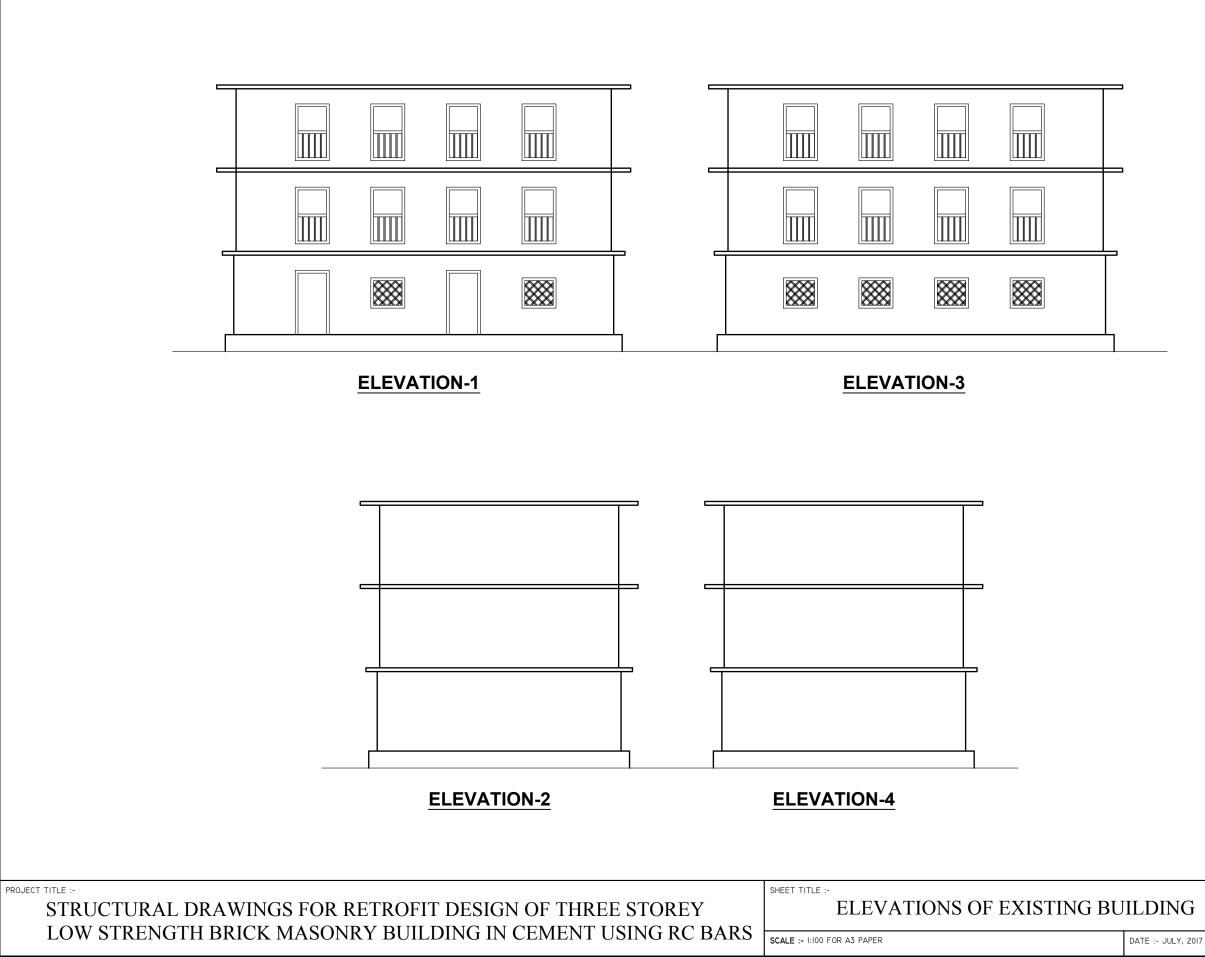
COVER PAGE

SCALE :- N/A

DATE :- JULY, 2017

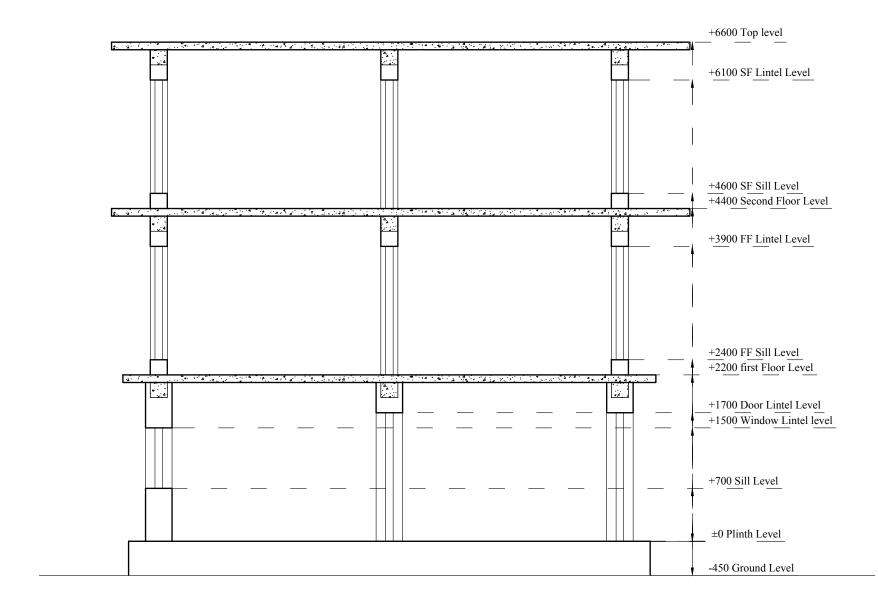
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OPENING SCHEDULE

S.N.	SYMBOL	NOS.	SIZE	SILL HEIGHT
1.	DOOR-D	10	900 x 1700	
2.	WINDOW-W1	6	900 x 800	700
3.	WINDOW-W2	16	900 x 1500	200



SECTION AT X-X

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF THREE STOREY LOW STRENGTH BRICK MASONRY BUILDING IN CEMENT USING RC BARS

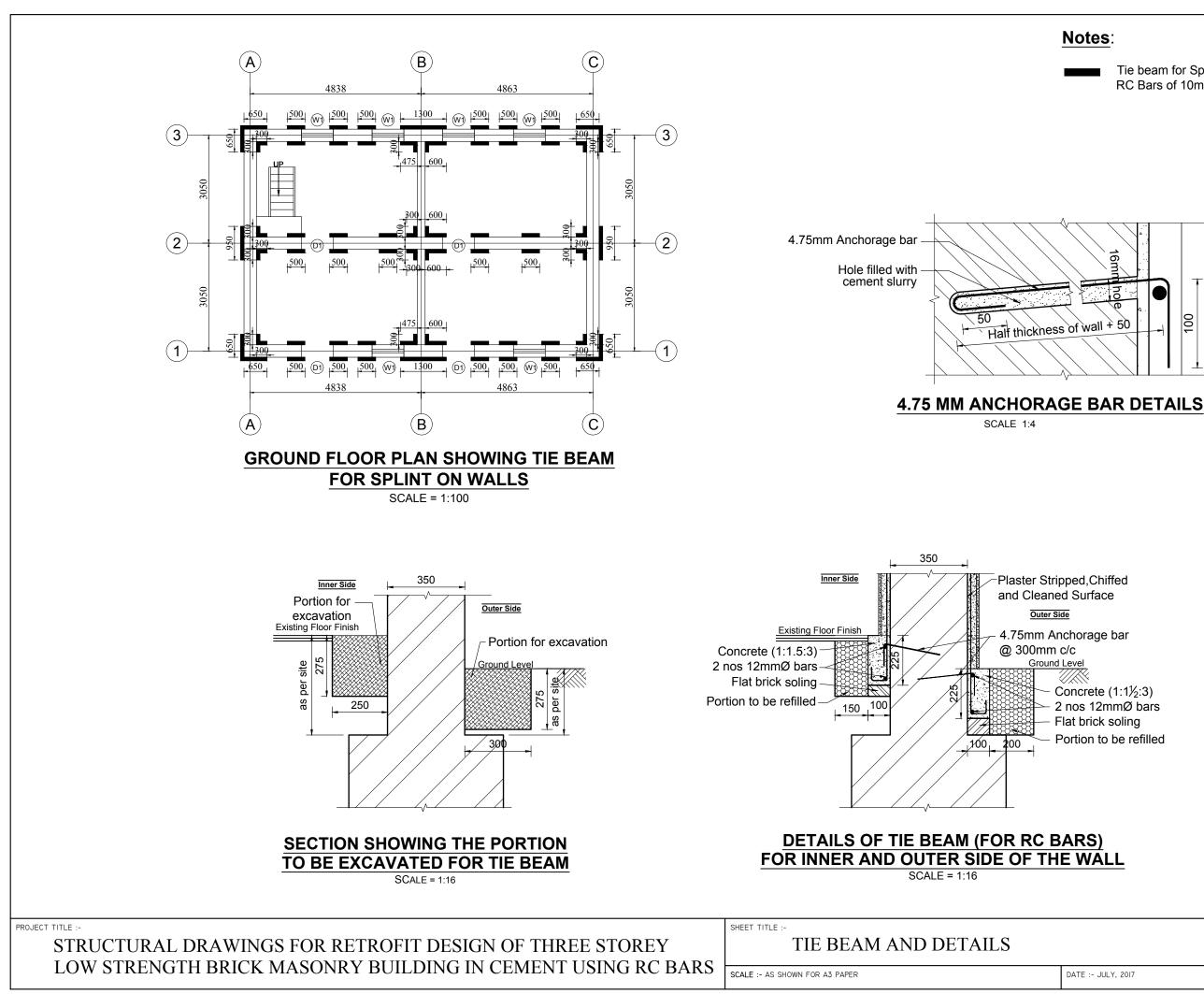
SECTION OF EXISTING BUILDING AND DETAILS

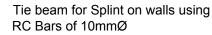
SCALE :- AS SHOWN FOR A3 PAPER

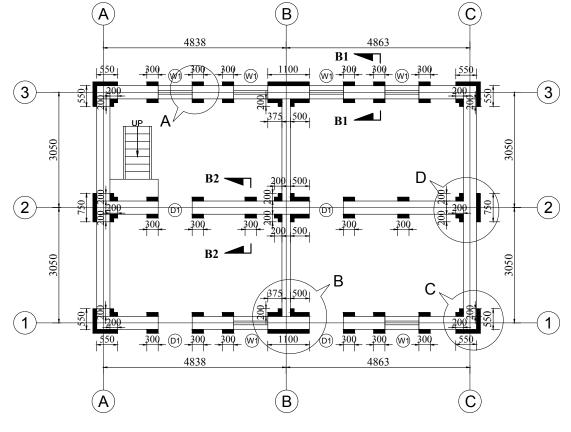
SHEET TITLE :-

03

DATE :- JULY, 2017







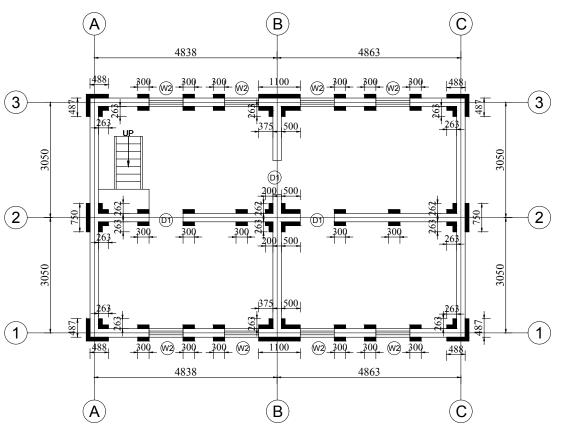


PROJECT TITLE :

SHEET TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF THREE STOREY LOW STRENGTH BRICK MASONRY BUILDING IN CEMENT USING RC BARS SCALE :- 1:100 FOR A3 PAPER

FLOOR PLANS SHOWING LOCATION OF SPLINT ON WALLS



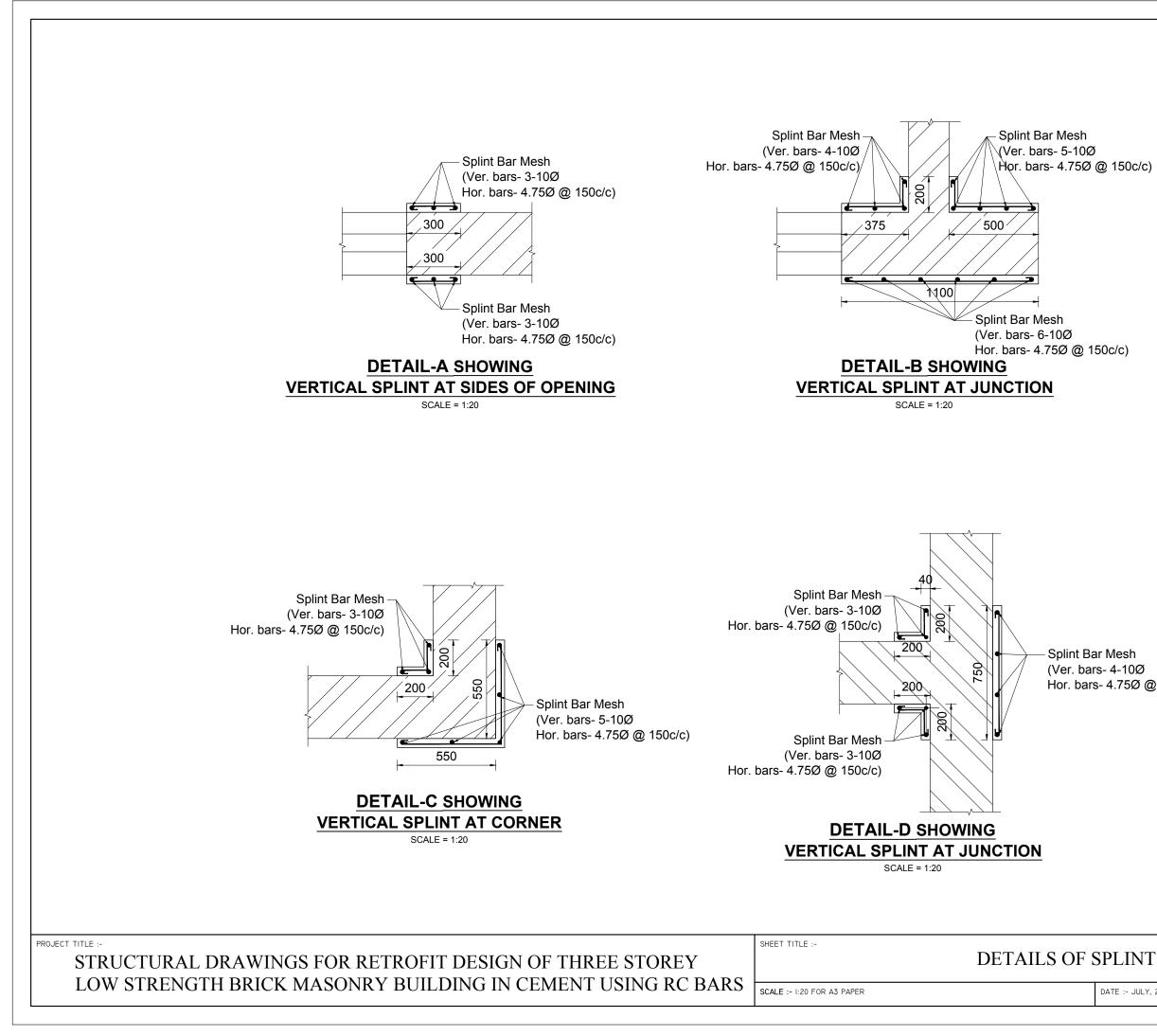
FIRST AND SECOND FLOOR PLAN SHOWING LOCATION OF SPLINT ON WALLS

Notes:

Splint

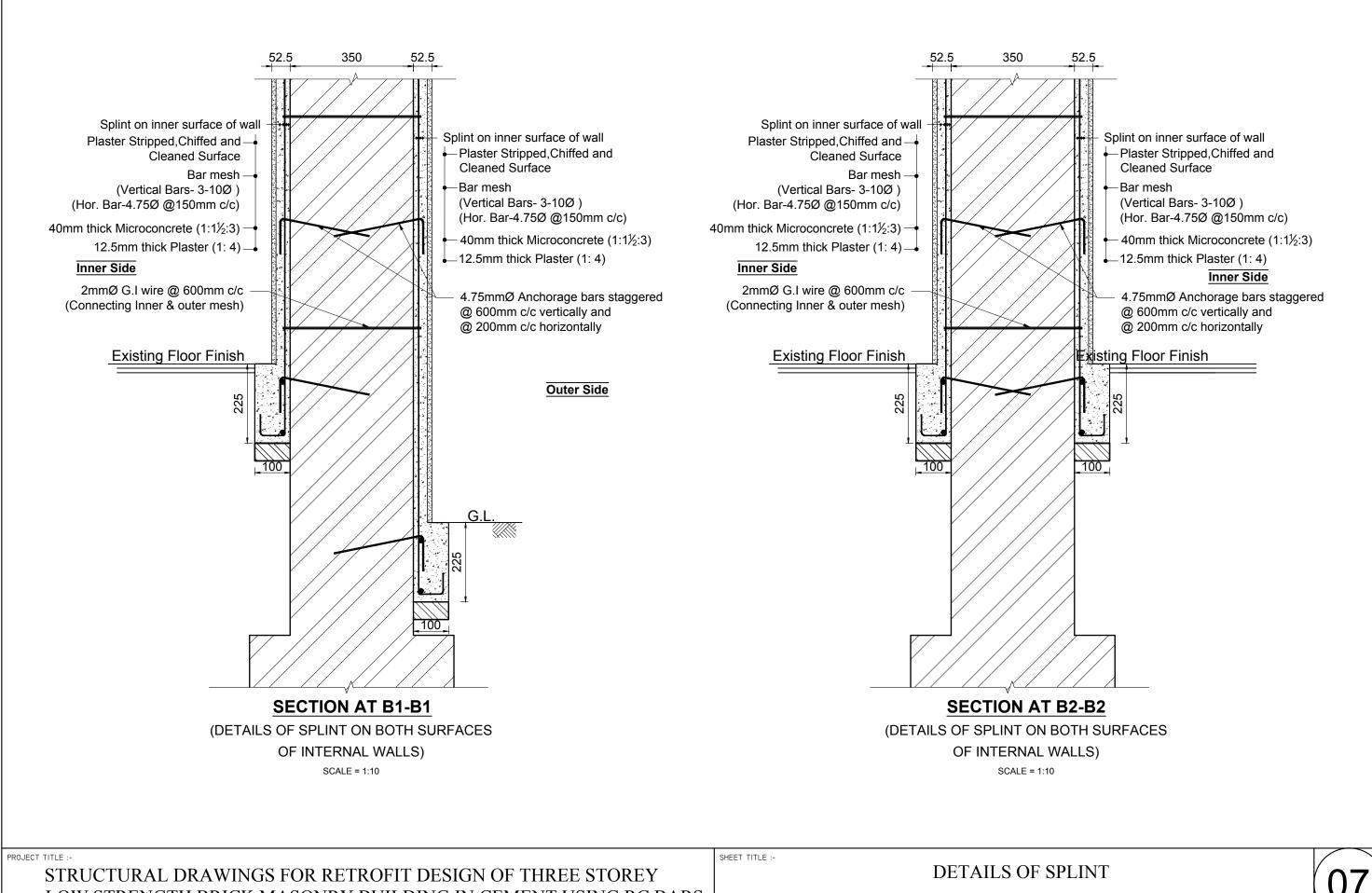
200mm width on each sides of corners and junctions using 3-10mmØ bars 300mm width for sides of openings and on walls using 3-10mmØ bars

05 DATE :- JULY, 2017



Hor. bars- 4.75Ø @ 150c/c)

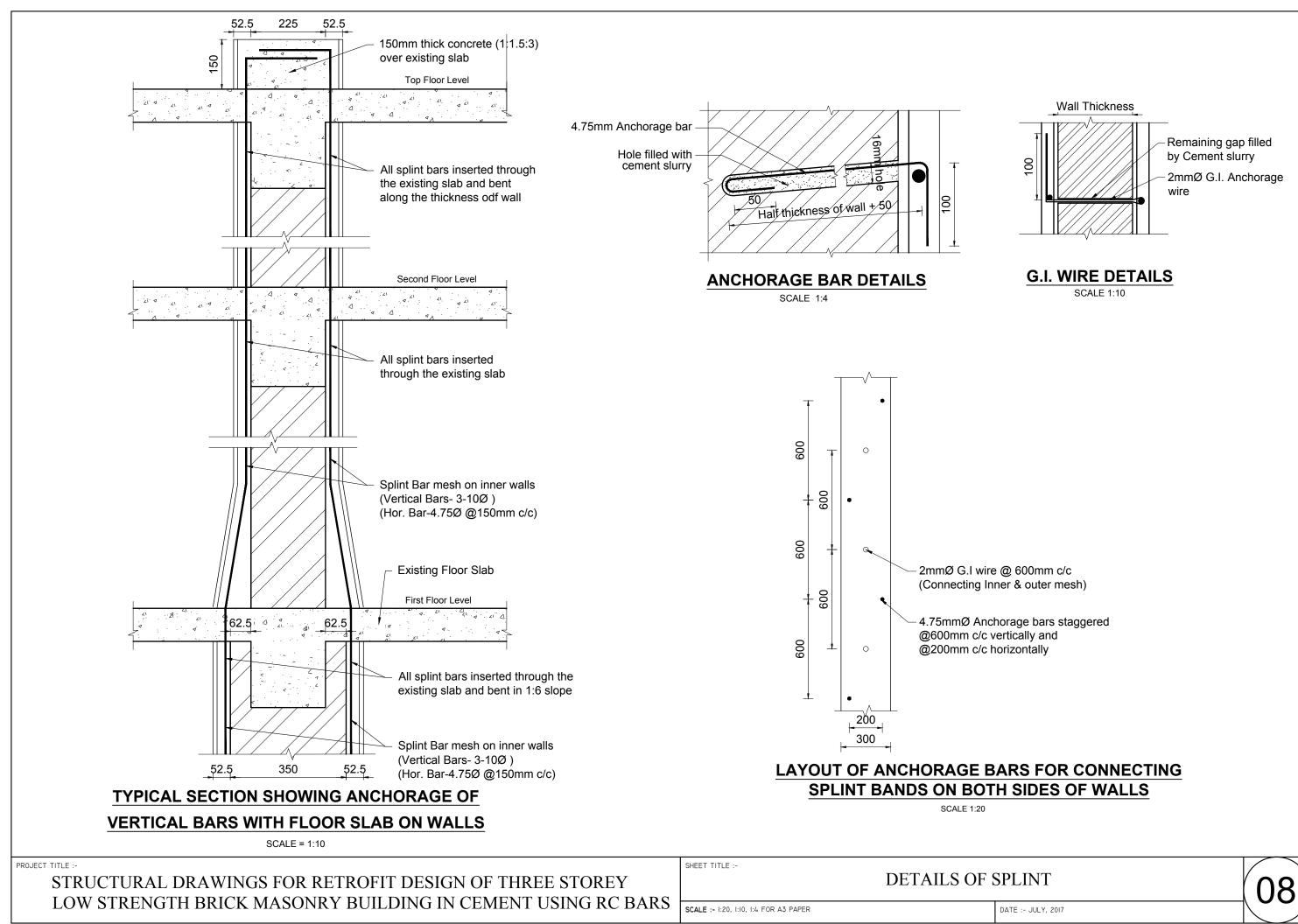
DATE :- JULY, 2017



LOW STRENGTH BRICK MASONRY BUILDING IN CEMENT USING RC BARS

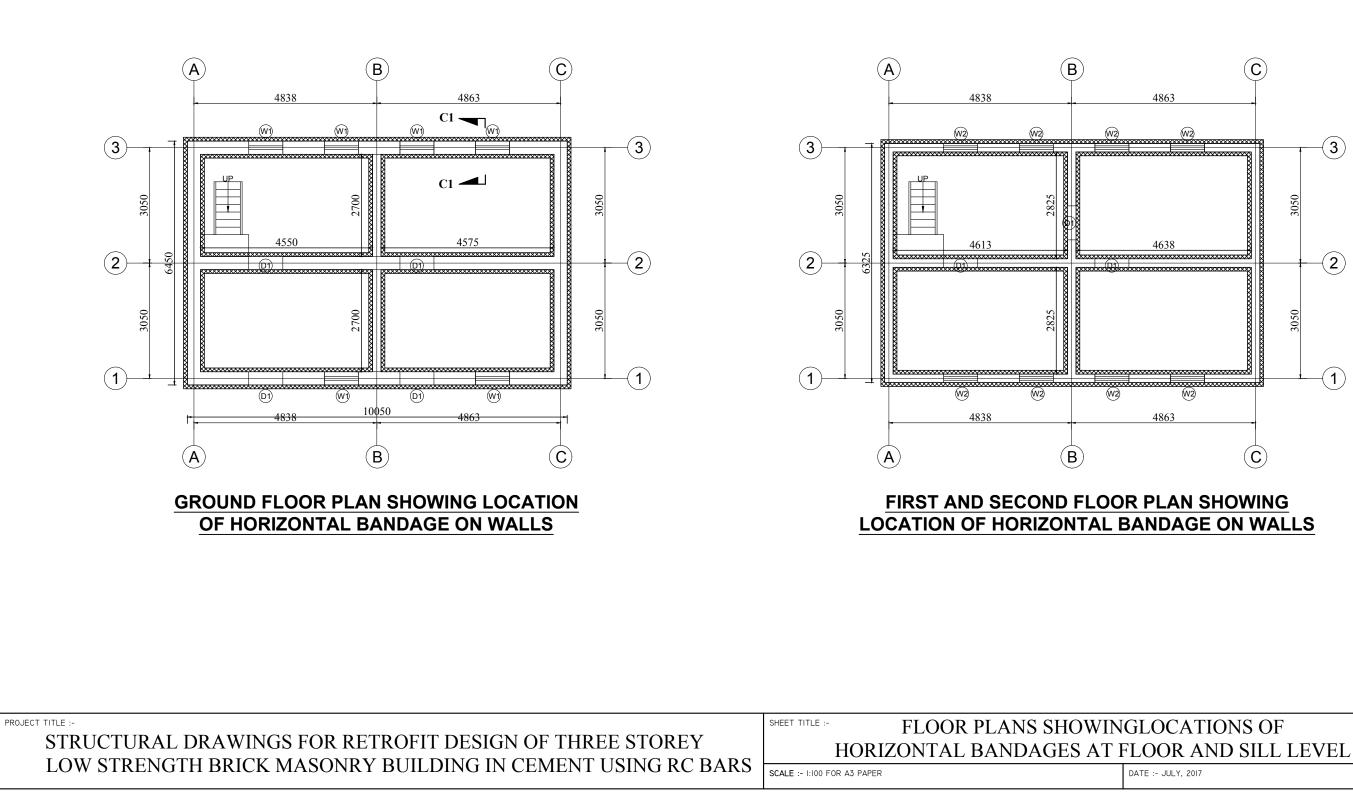
SCALE :- 1:10 FOR A3 PAPER

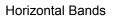
DATE :- JULY, 2017

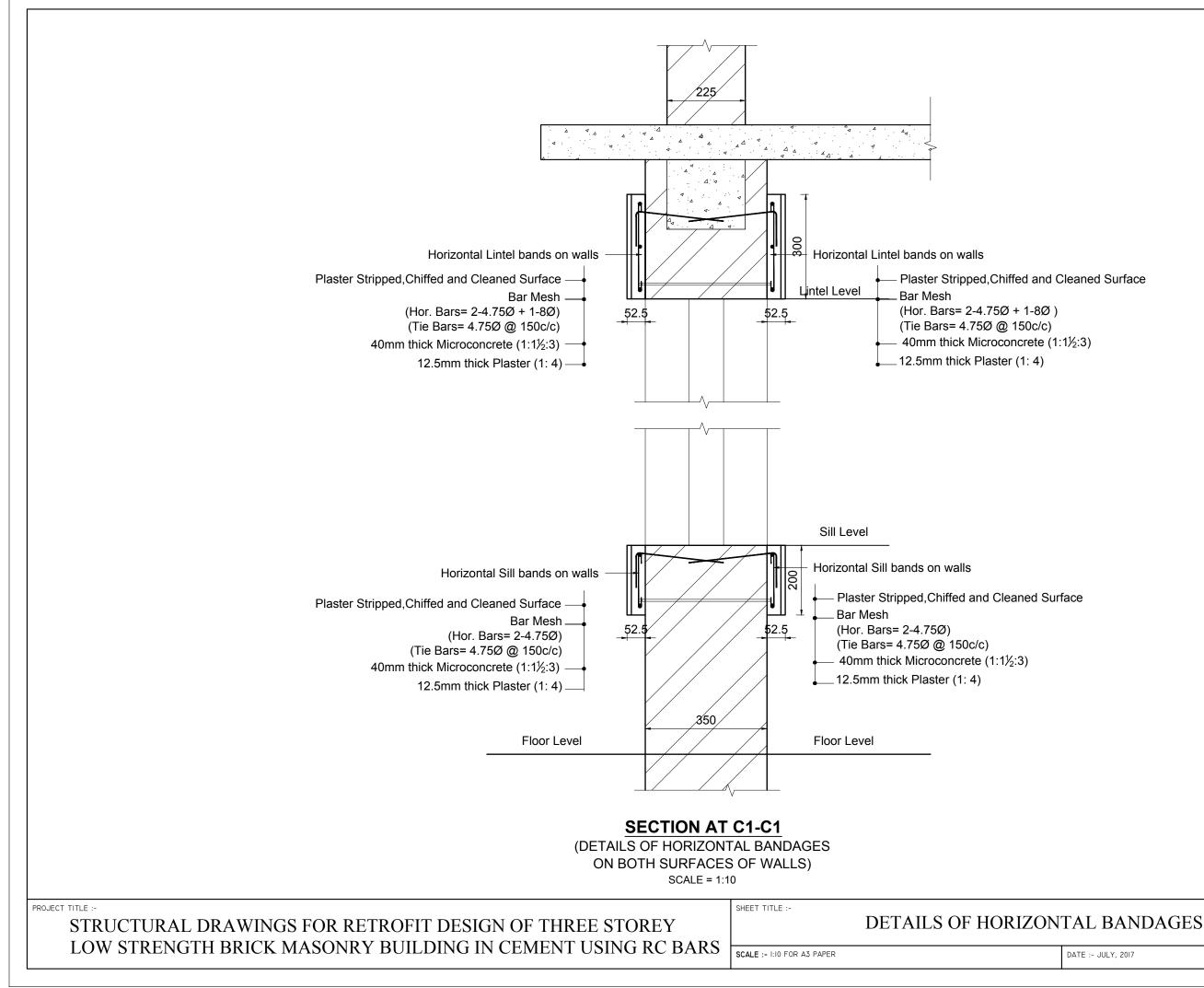


Notes:

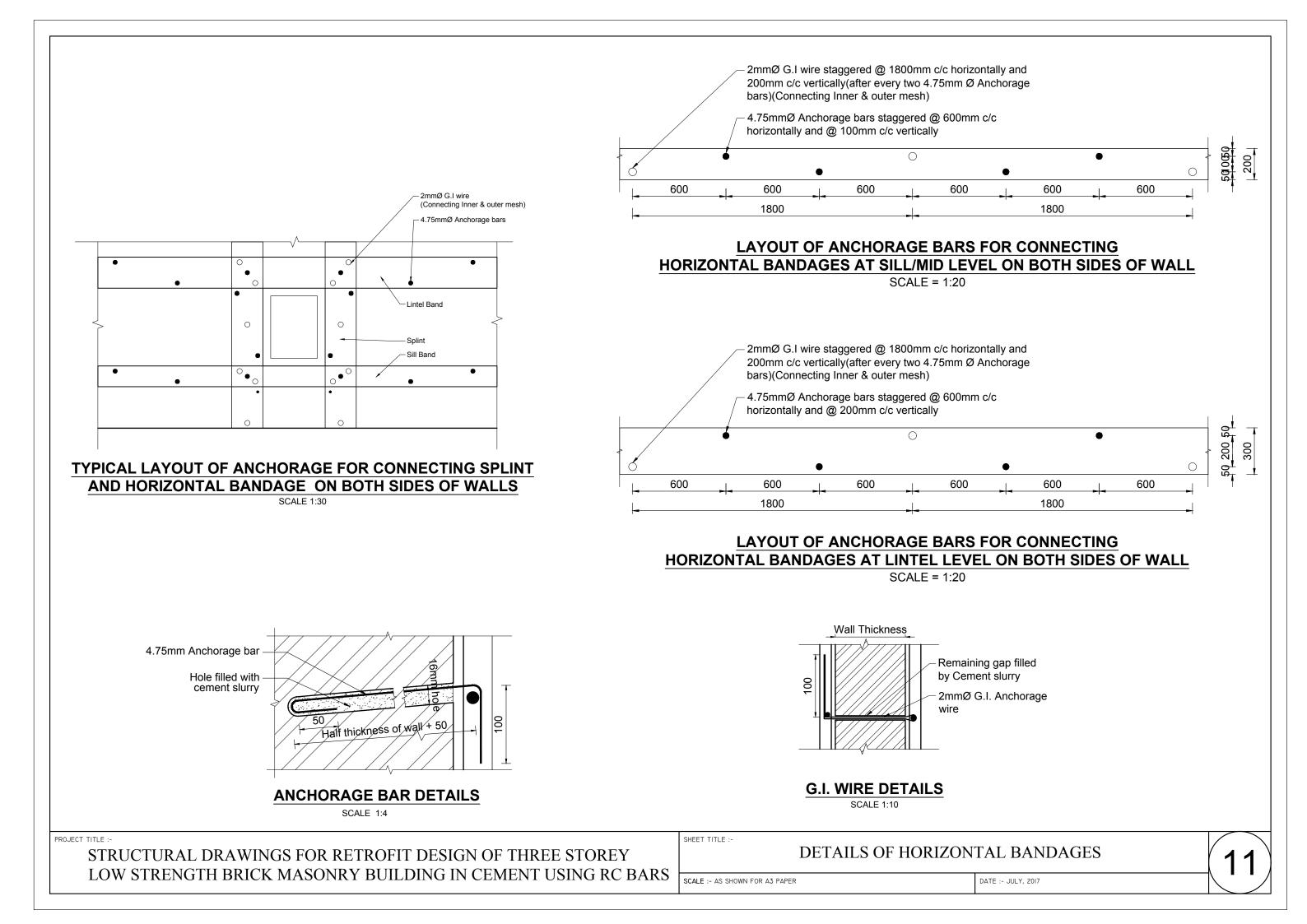
200mm width using 2-4.75mmØ bars at mid level 300mm width using 2-4.75mmØ + 1-8mmØ bars at lintel level

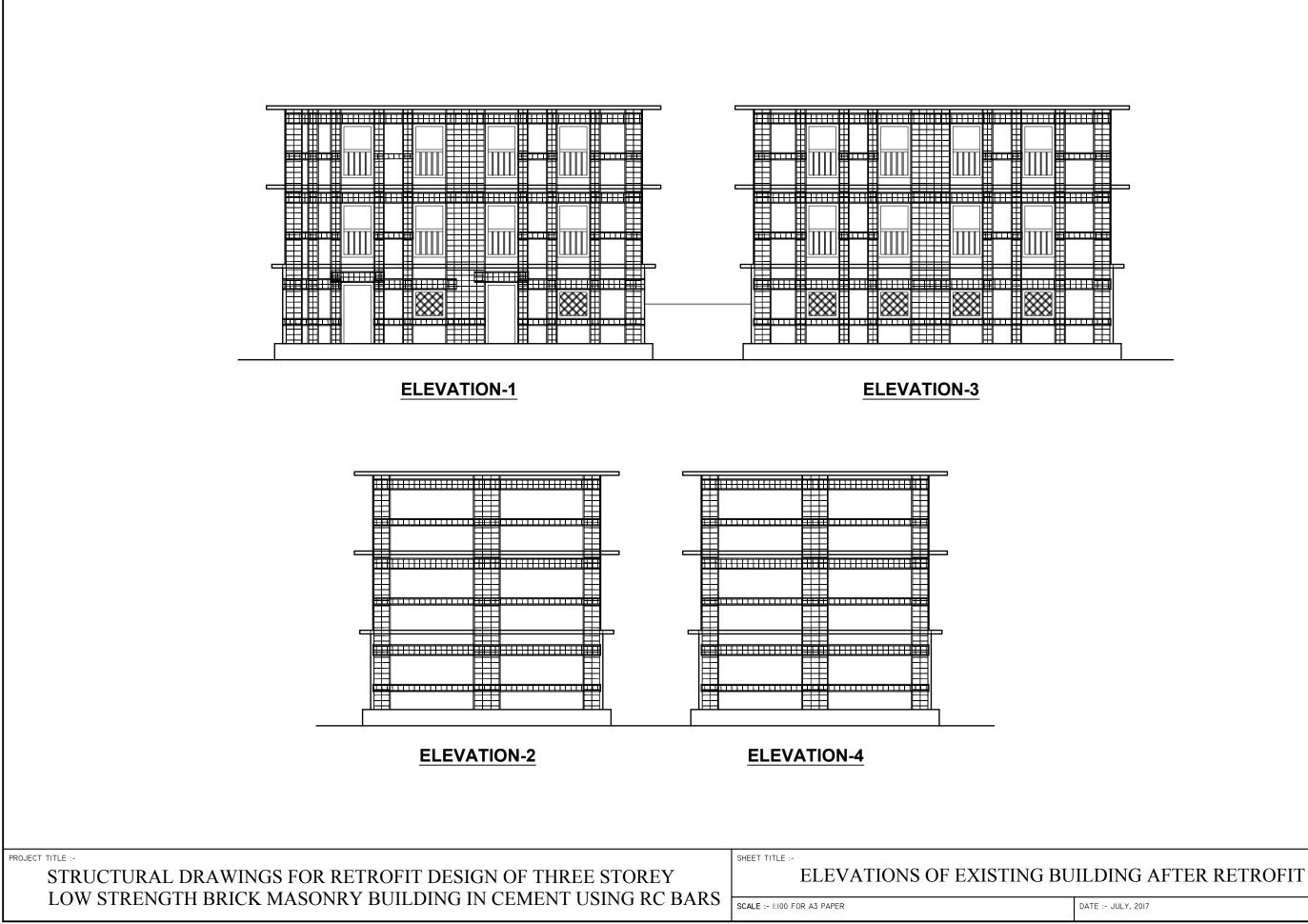






DATE :- JULY, 2017





STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON TWO STOREY BRICK MASONRY BUILDING IN MUD USING RC BARS

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH BRICK MASONRY BUILDING IN MUD USING RC BARS

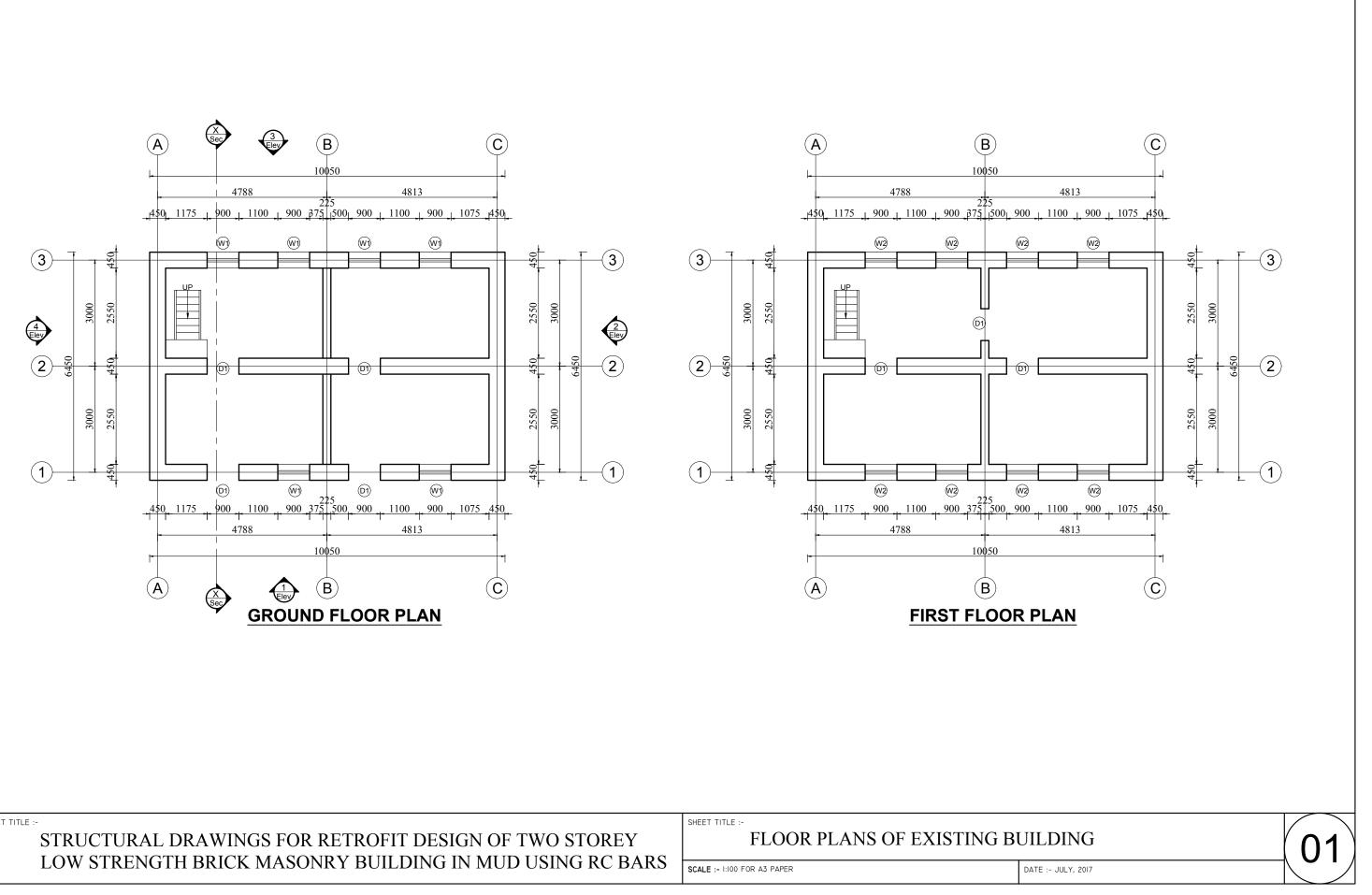
SHEET TITLE :-

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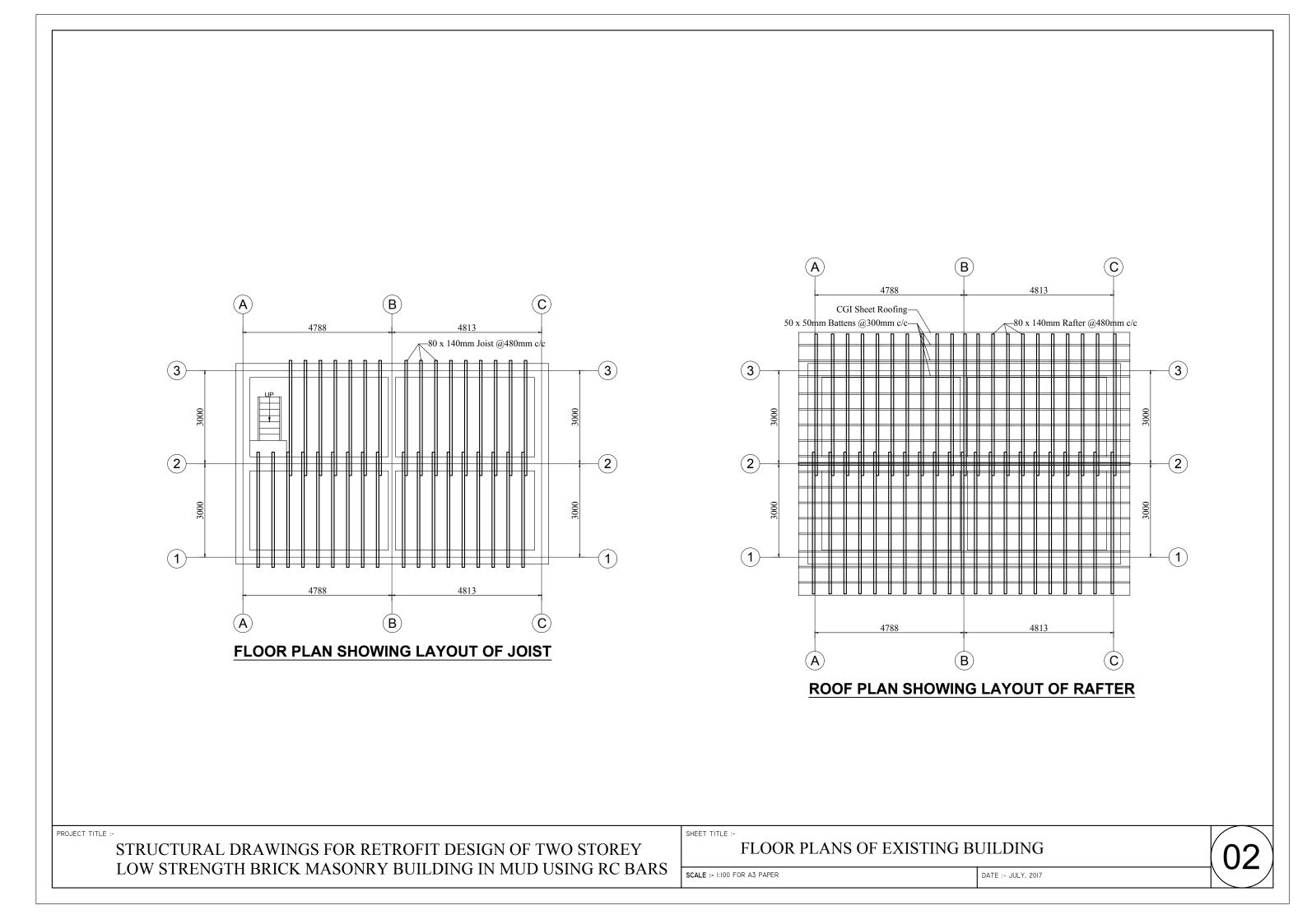
SCALE :- N/A

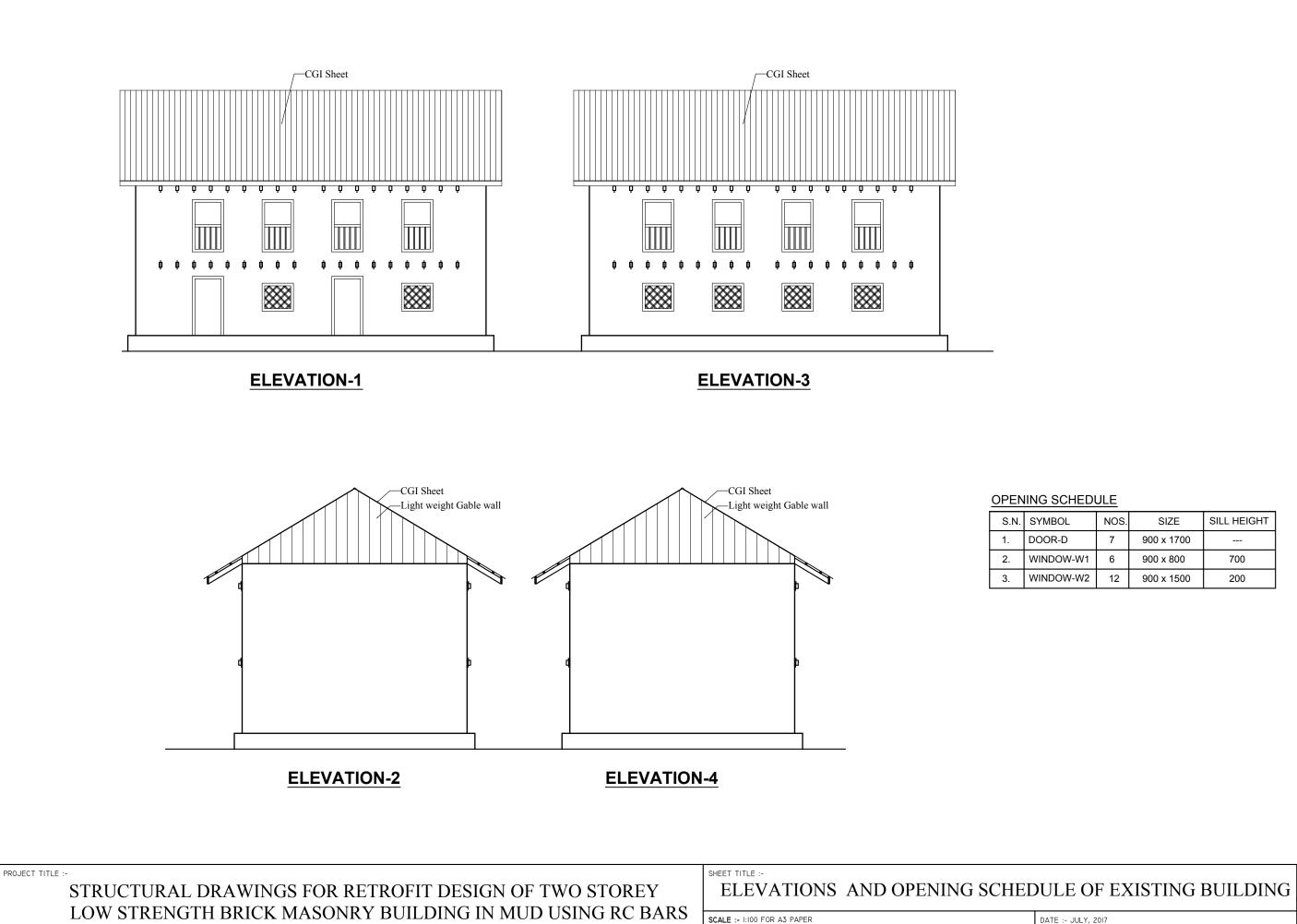
DATE :- JULY, 2017

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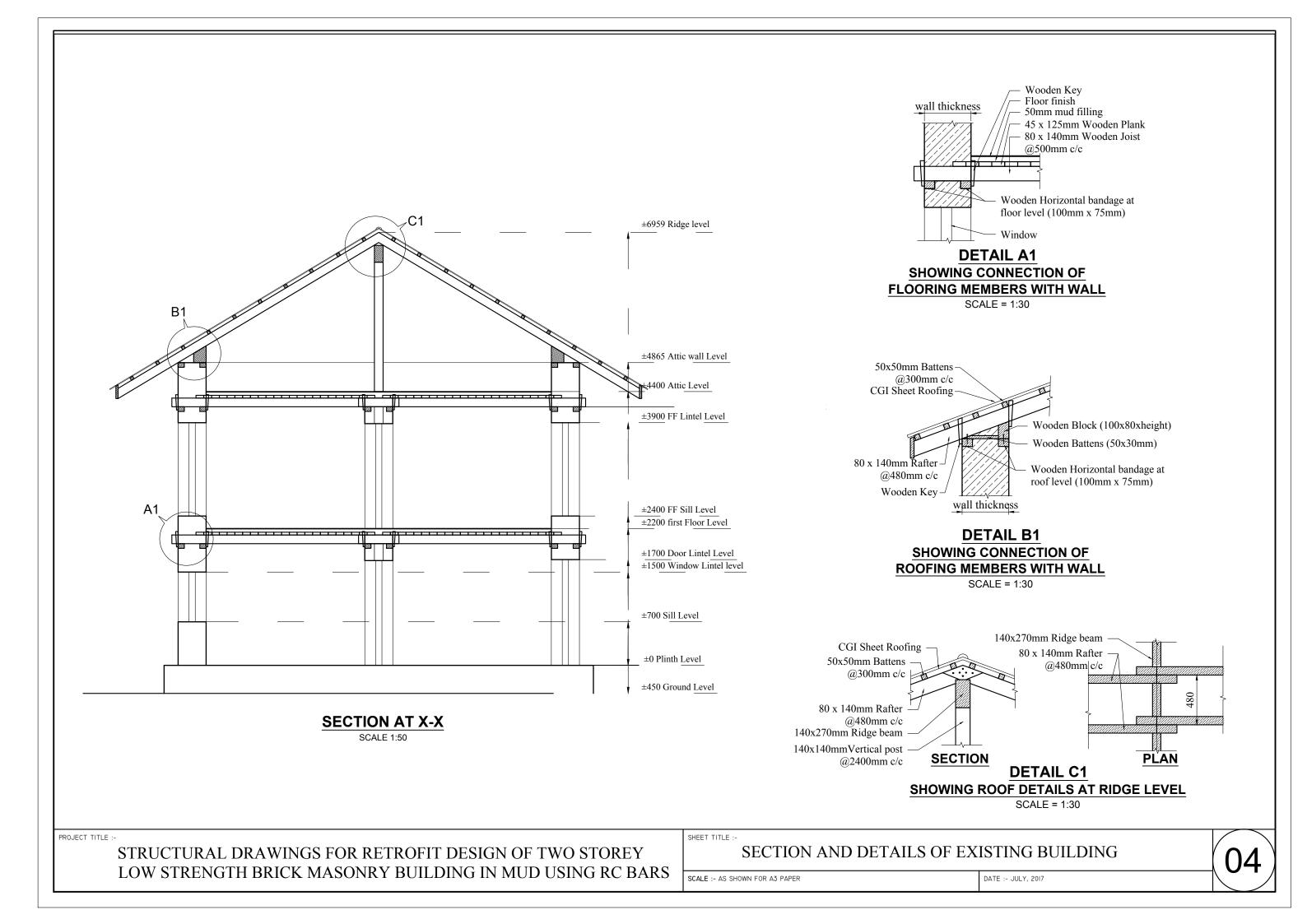
PROJECT TITLE :-

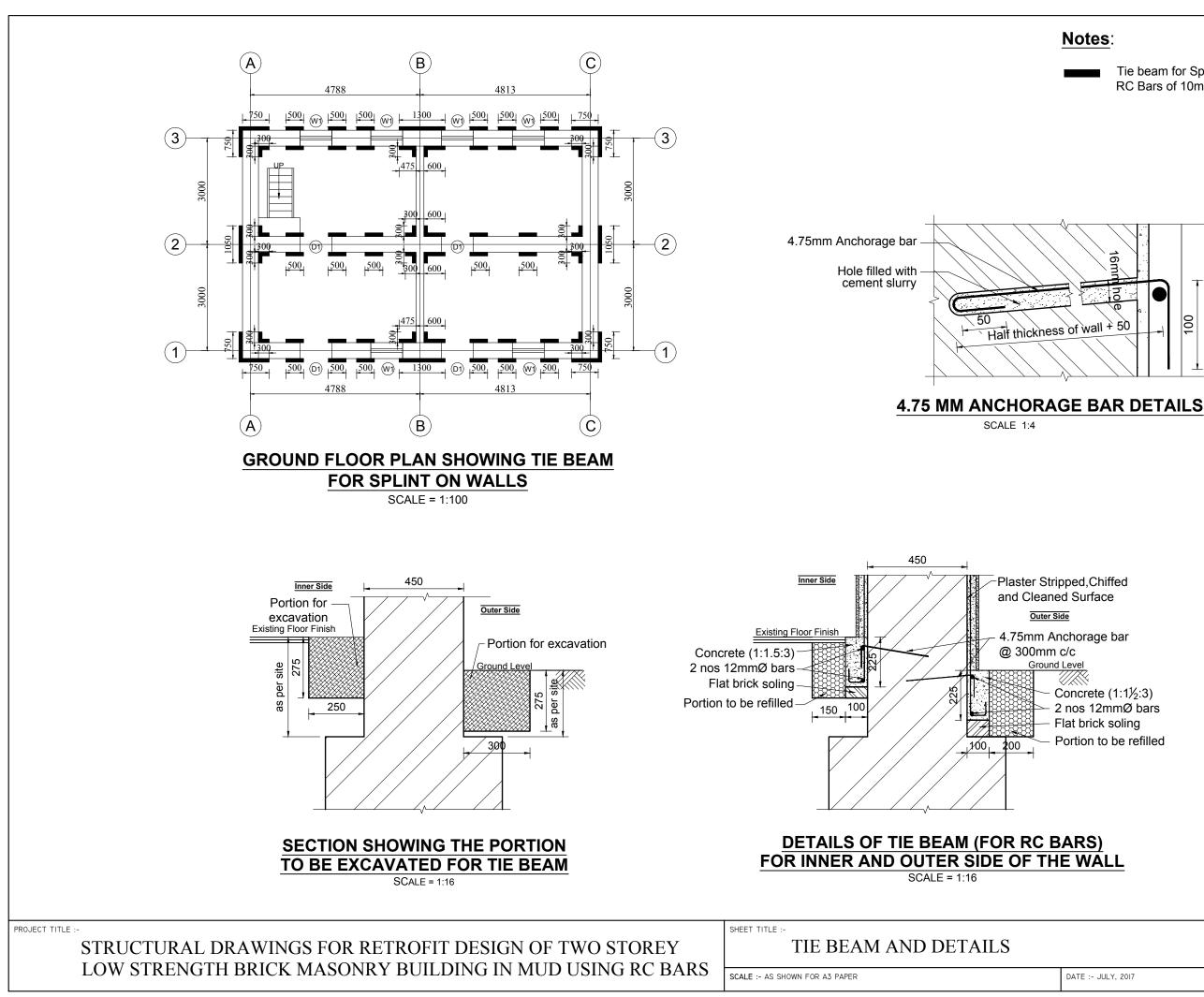


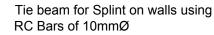


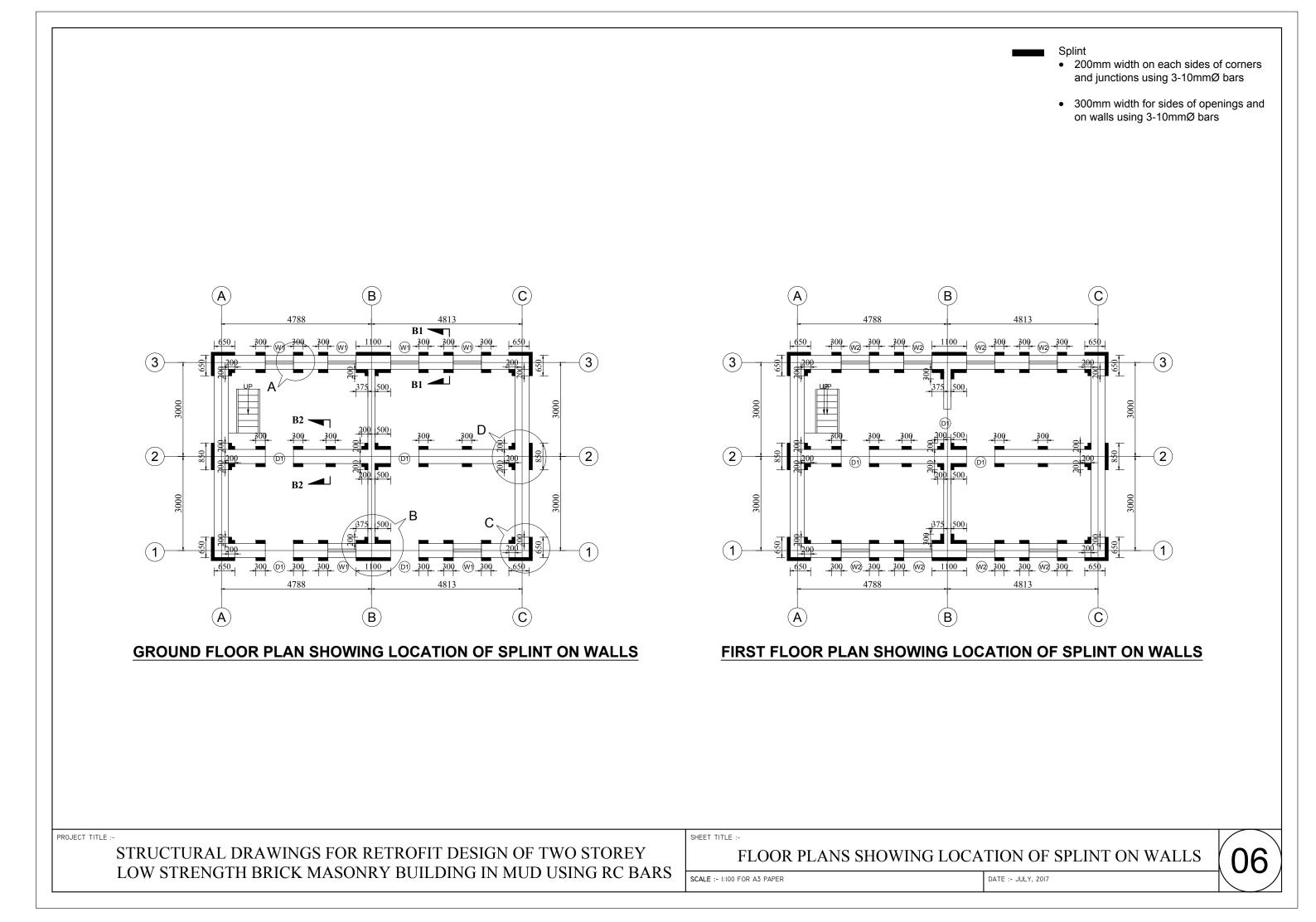
MBOL	NOS.	SIZE	SILL HEIGHT
OR-D	7	900 x 1700	
NDOW-W1	6	900 x 800	700
NDOW-W2	12	900 x 1500	200

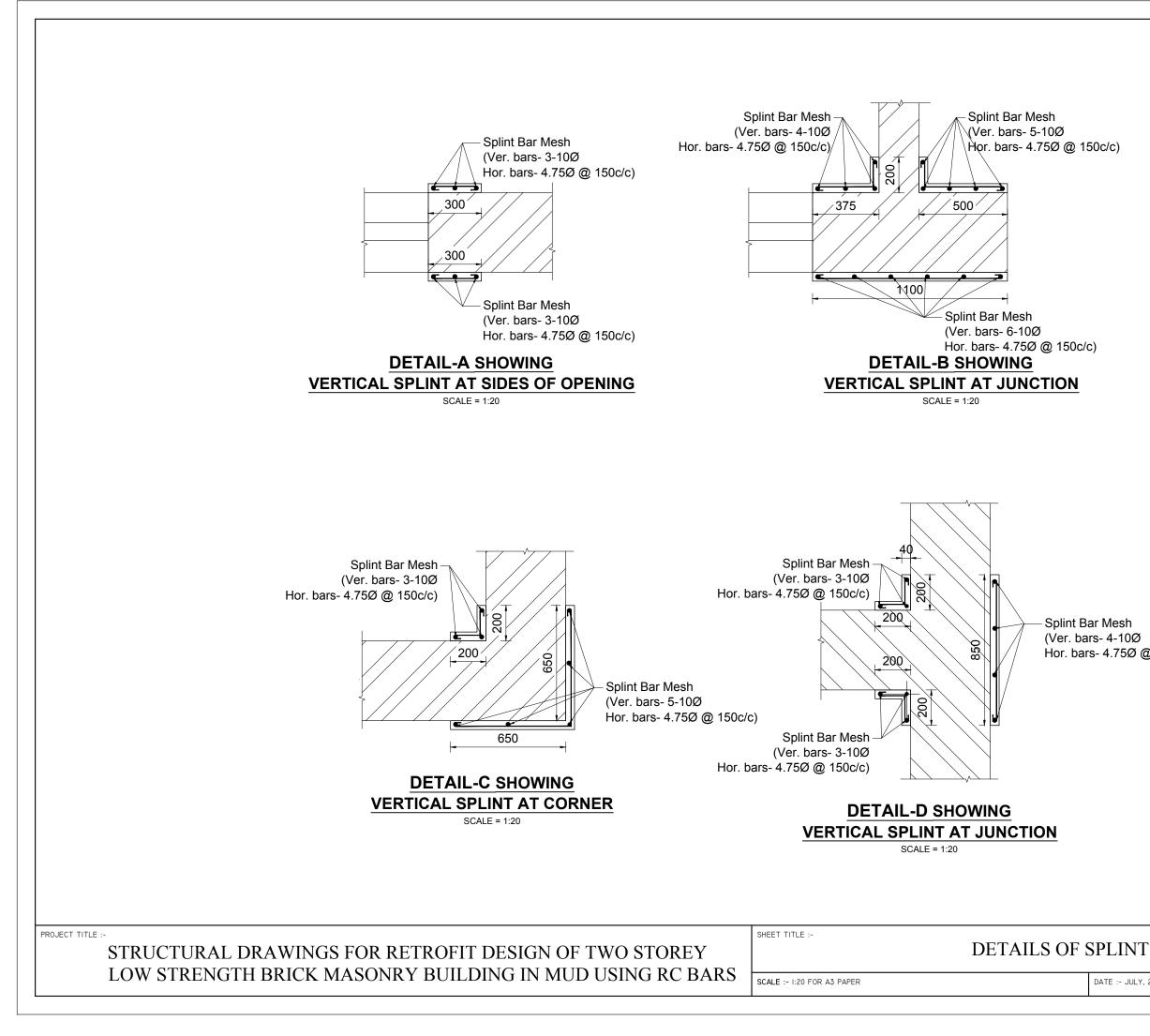
DATE :- JULY, 2017





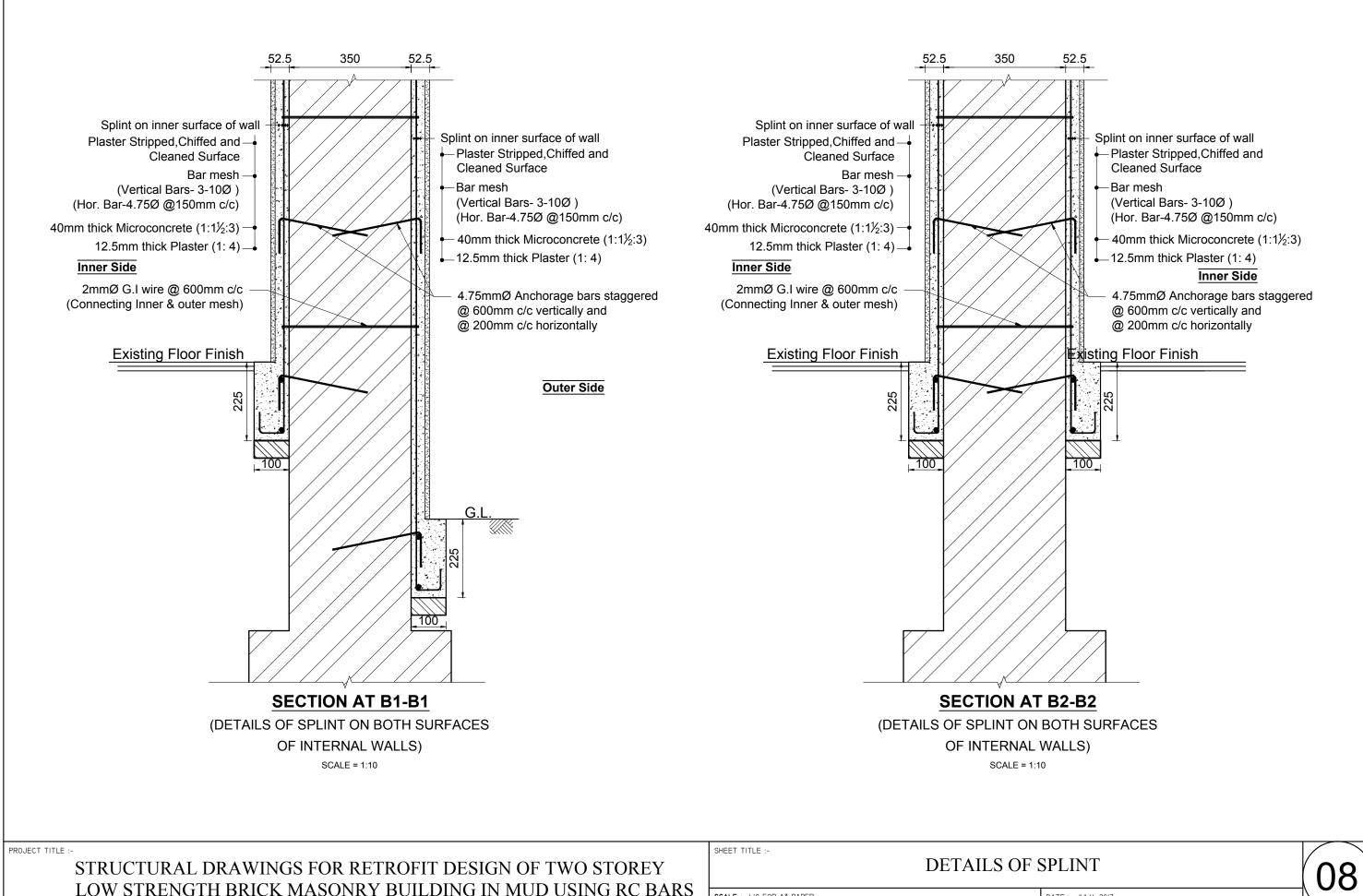




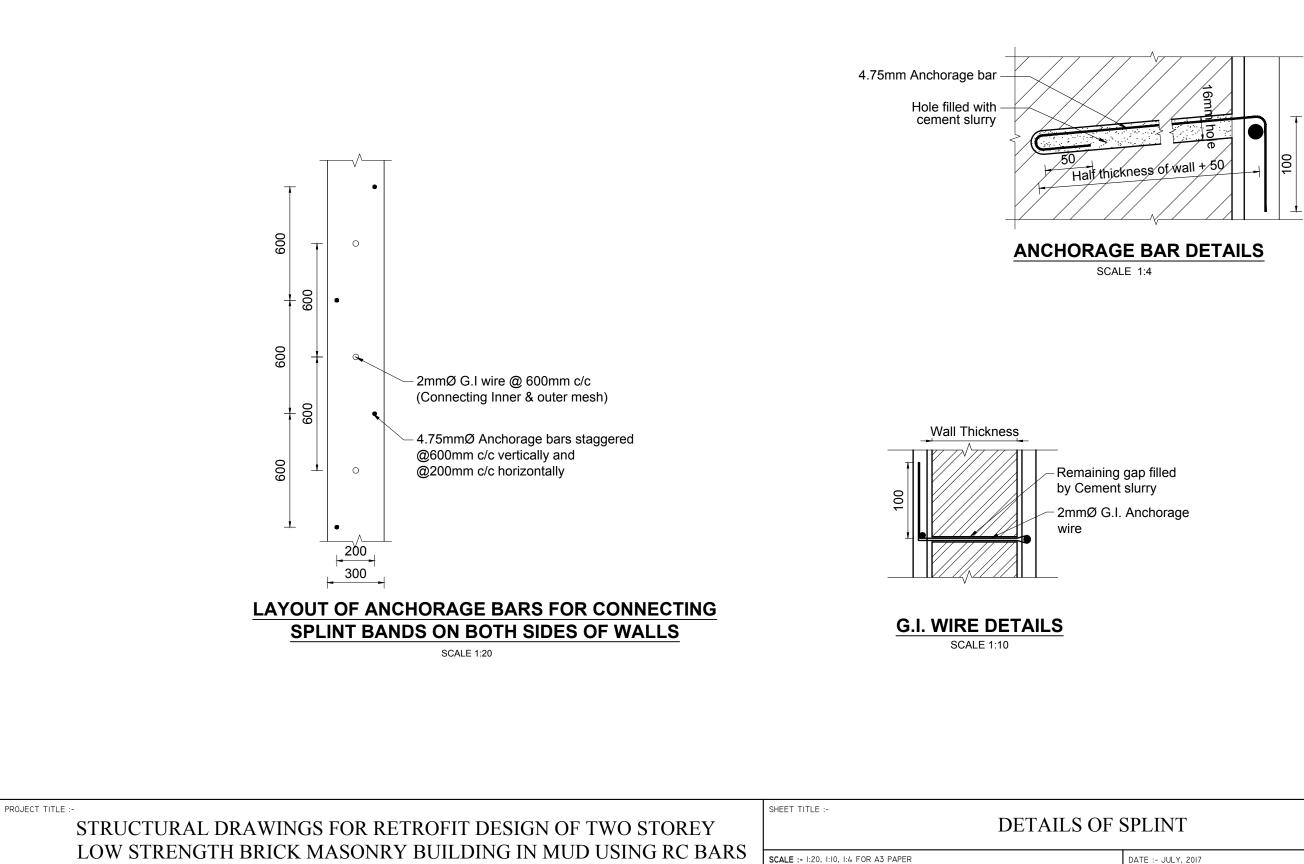


Hor. bars- 4.75Ø @ 150c/c)

DATE :- JULY, 2017



SCALE :- 1:10 FOR A3 PAPER

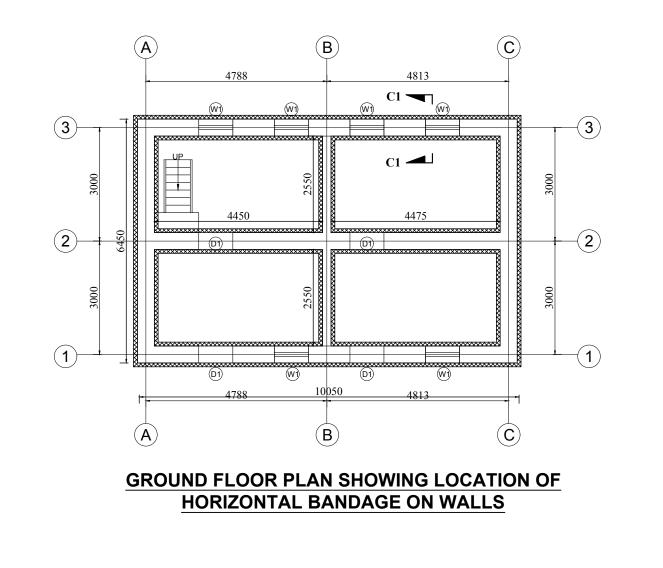


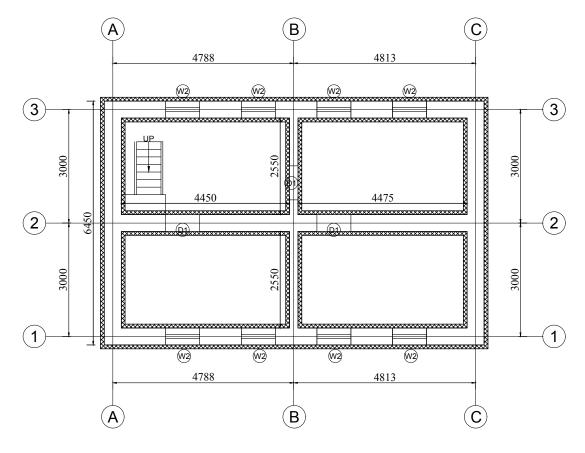
DATE :- JULY, 2017

Notes:

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FIRST FLOOR PLAN SHOWING LOCATION OF HORIZONTAL BANDAGE ON WALLS

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH BRICK MASONRY BUILDING IN MUD USING RC BARS

SHEET TITLE :-FLOOR PLANS SHOWINGLOCATIONS OF

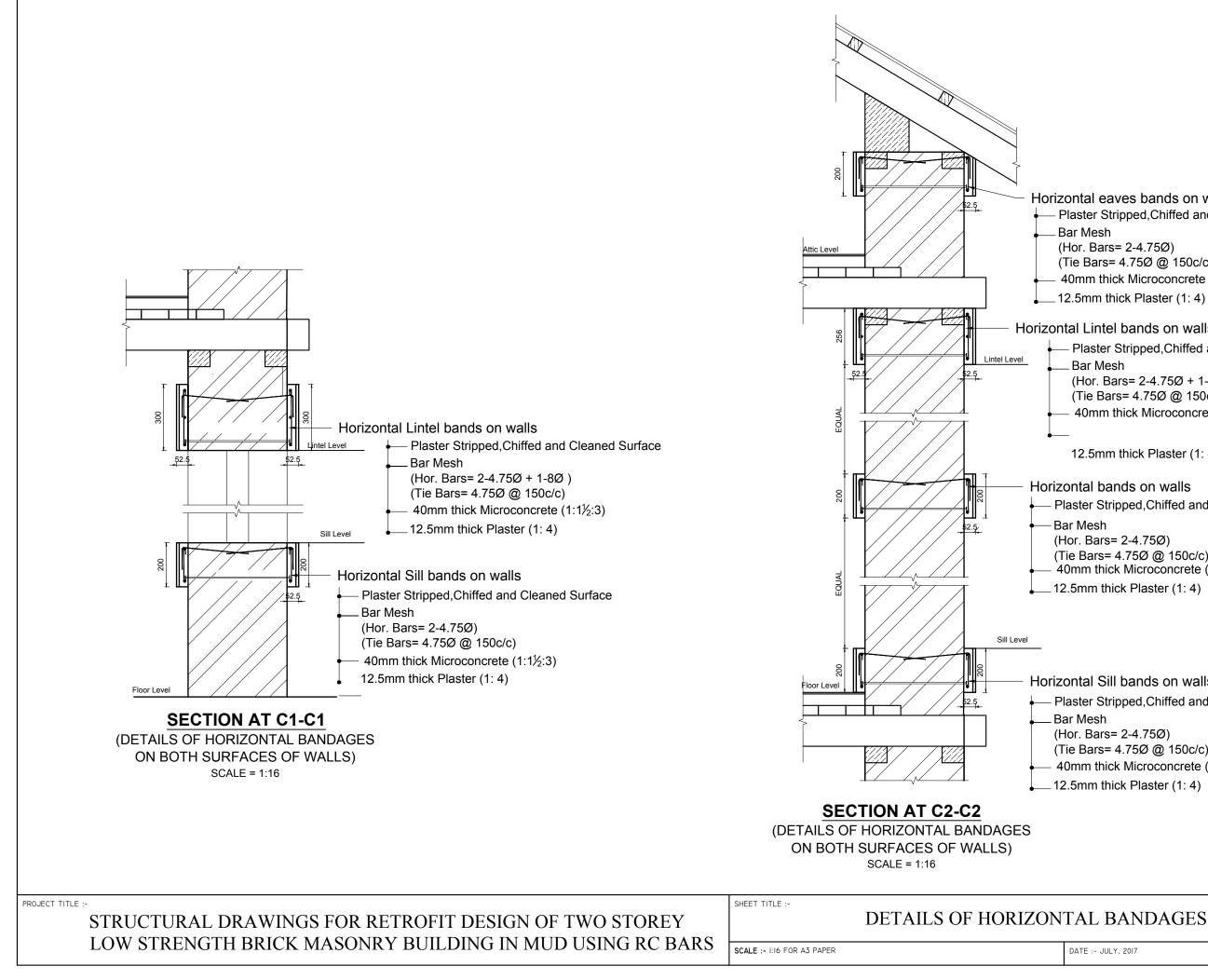
HORIZONTAL BANDAGES AT FLOOR AND SILL LEVEL

SCALE :- I:100 FOR A3 PAPER

Horizontal Bands

200mm width using 2-4.75mmØ bars at sill and mid level 300mm width using 2-4.75mmØ + 1-8mmØ bars at lintel level

10



Horizontal eaves bands on walls - Plaster Stripped, Chiffed and Cleaned Surface Bar Mesh (Hor. Bars= 2-4.75Ø) (Tie Bars= 4.75Ø @ 150c/c) 40mm thick Microconcrete $(1:1\frac{1}{2}:3)$ 12.5mm thick Plaster (1: 4)

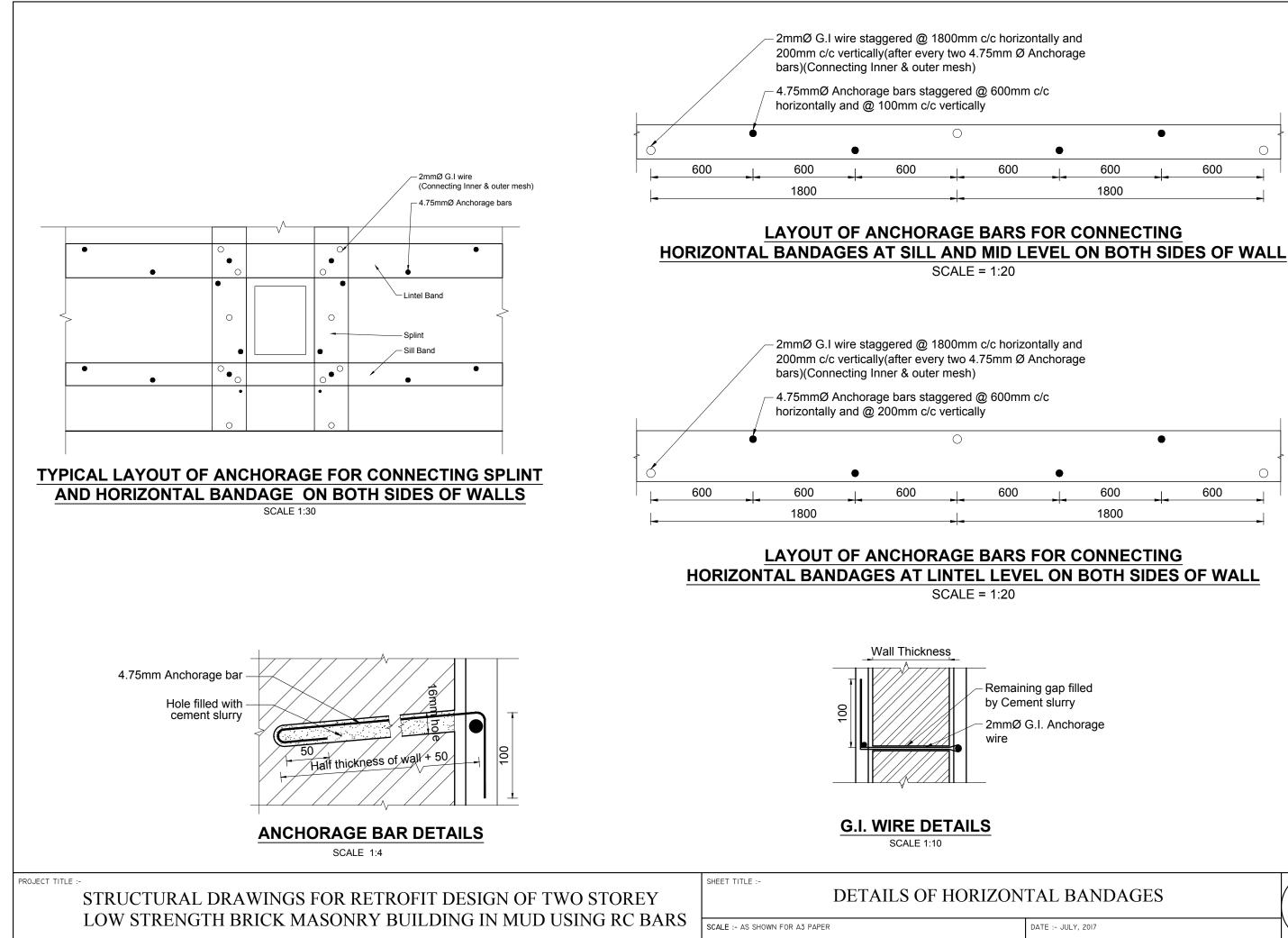
Horizontal Lintel bands on walls Plaster Stripped, Chiffed and Cleaned Surface Bar Mesh (Hor. Bars= 2-4.75Ø + 1-8Ø) (Tie Bars= 4.75Ø @ 150c/c) 40mm thick Microconcrete $(1:1\frac{1}{2}:3)$

12.5mm thick Plaster (1: 4)

Horizontal bands on walls Plaster Stripped, Chiffed and Cleaned Surface Bar Mesh (Hor. Bars= 2-4.75Ø) (Tie Bars= 4.75Ø @ 150c/c) 40mm thick Microconcrete (1:1½:3) 12.5mm thick Plaster (1: 4)

Horizontal Sill bands on walls - Plaster Stripped, Chiffed and Cleaned Surface Bar Mesh (Hor. Bars= 2-4.75Ø) (Tie Bars= 4.75Ø @ 150c/c) 40mm thick Microconcrete $(1:1\frac{1}{2}:3)$ 12.5mm thick Plaster (1: 4)

1



. • 50 200 50 Ο Remaining gap filled 2mmØ G.I. Anchorage DATE :- JULY, 2017

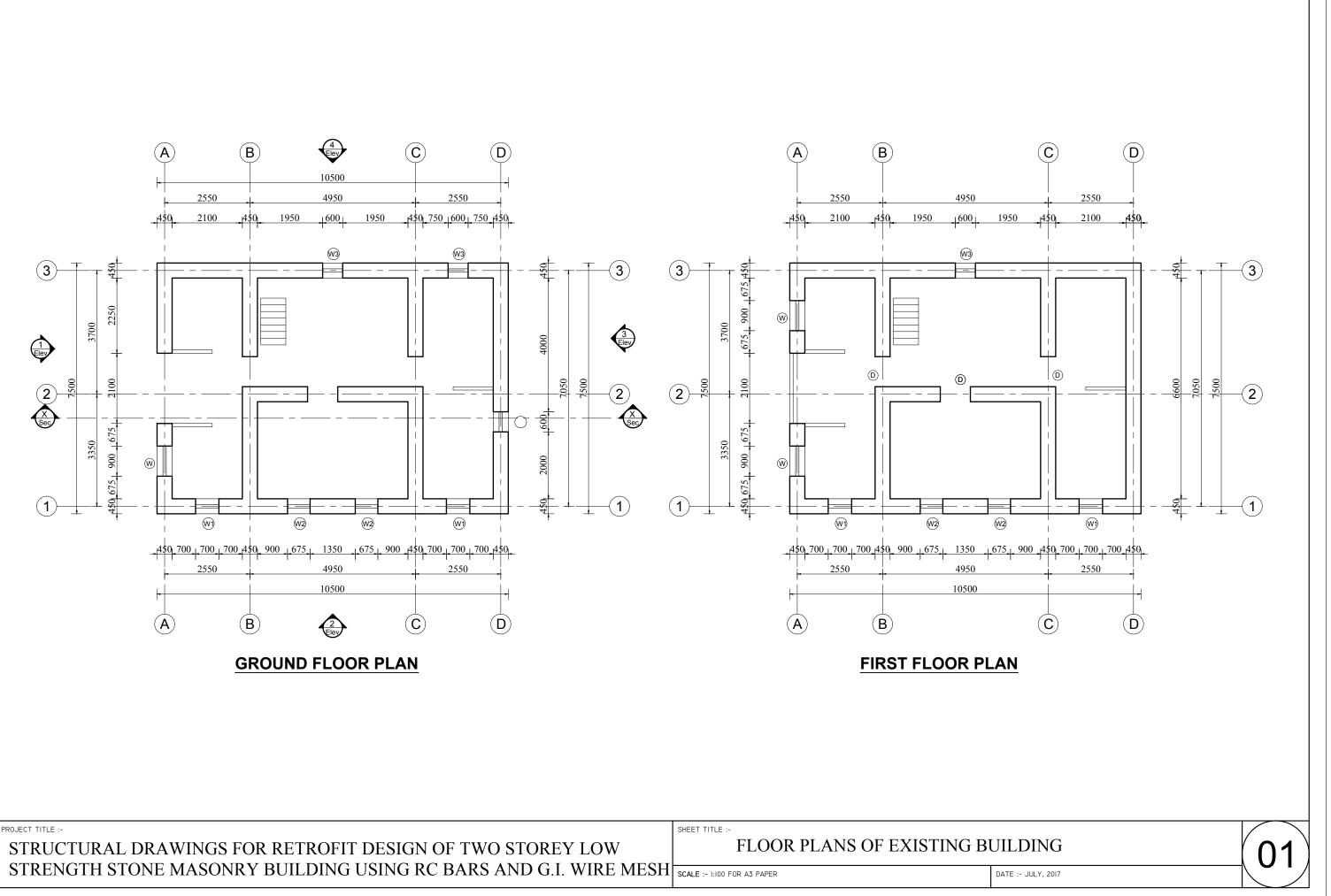
STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON **TWO STOREY STONE MASONRY BUILDING IN MUD USING RC BARS AND G.I. WIREMESH**

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING RC BARS AND G.I. WIRE MESH

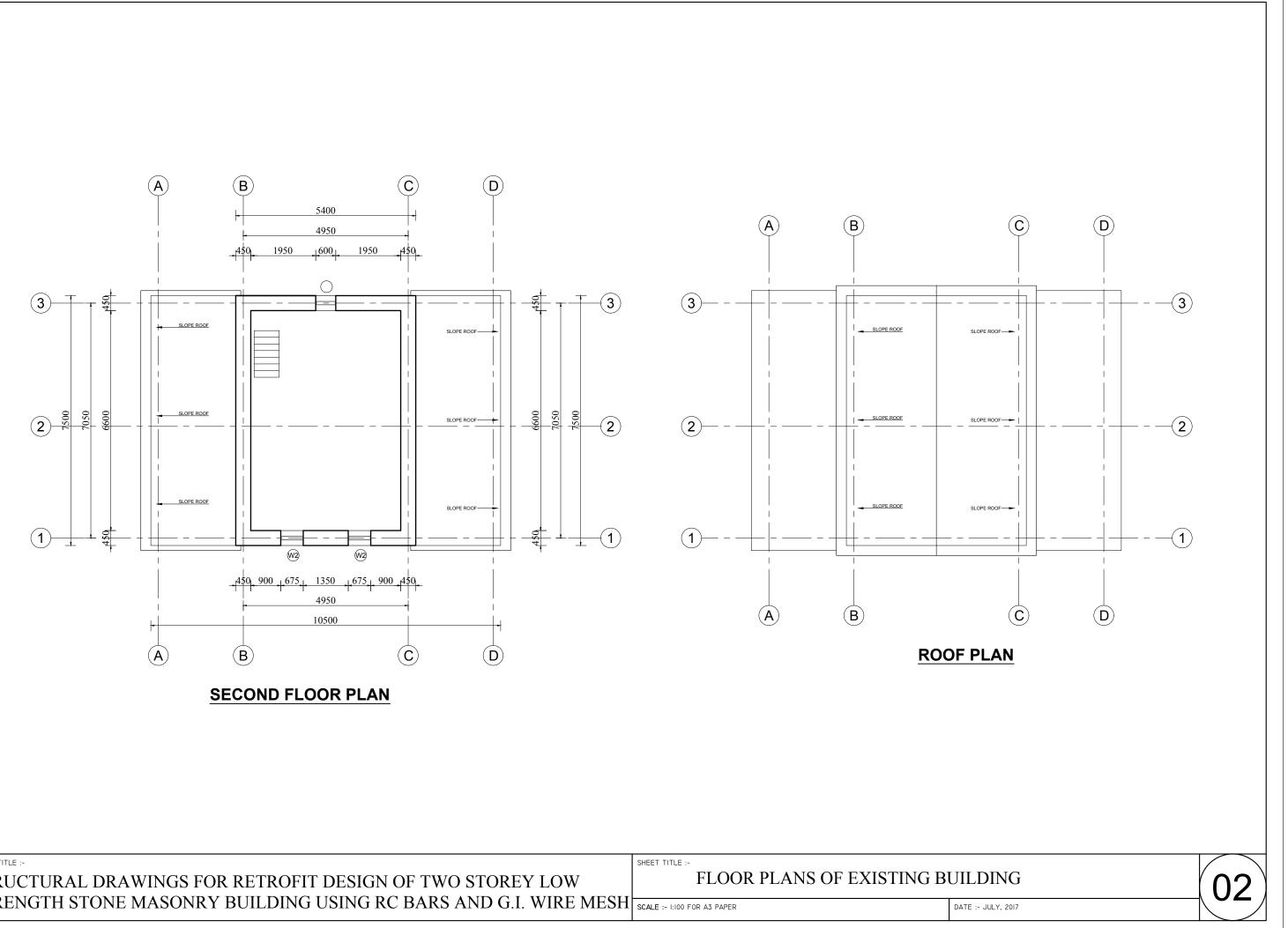
SHEET TITLE :-COVER PAGE

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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING RC BARS AND G.I. WIRE MESH

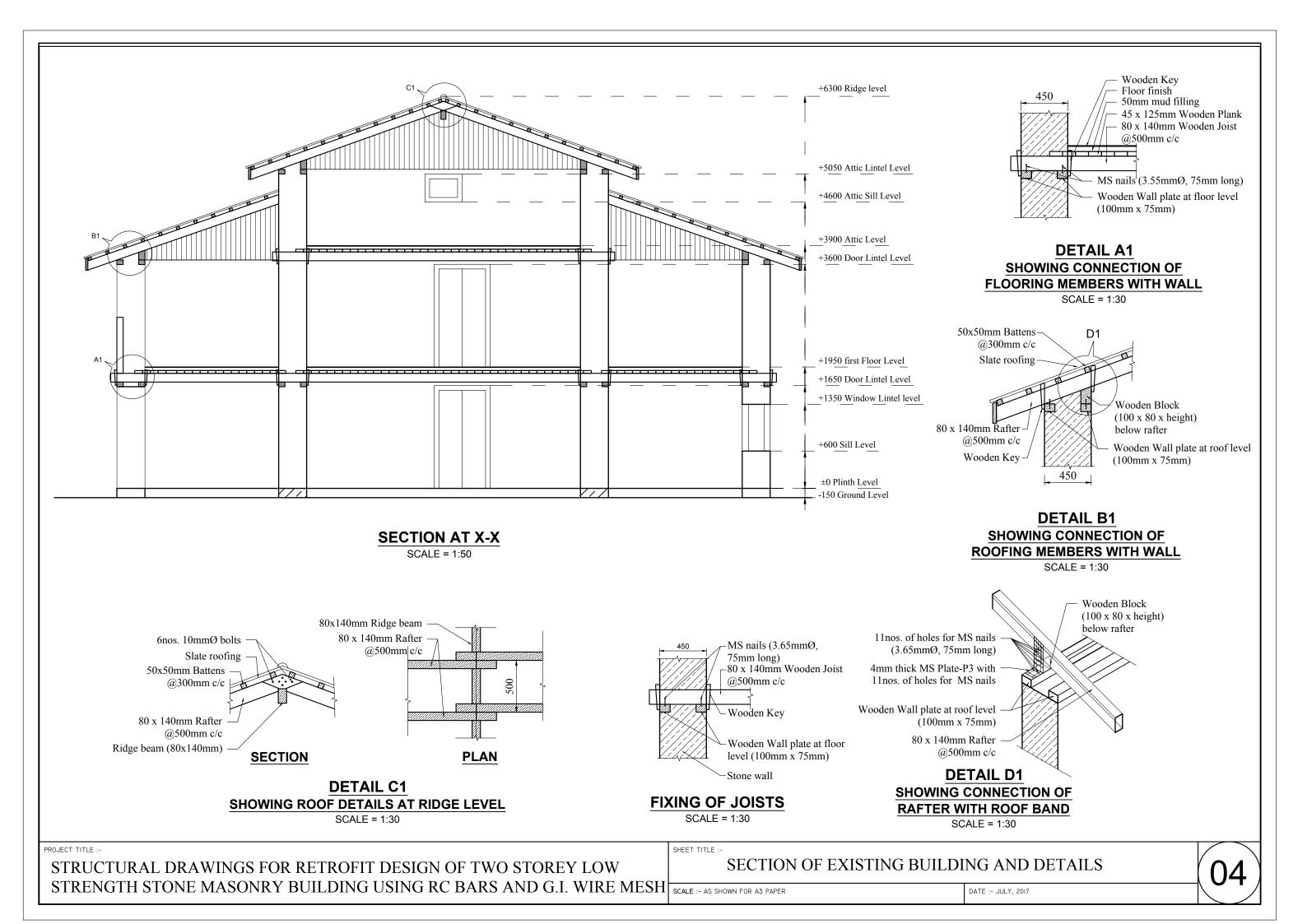
PROJECT TITLE :-STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING RC BARS AND G.I. WIRE MESH

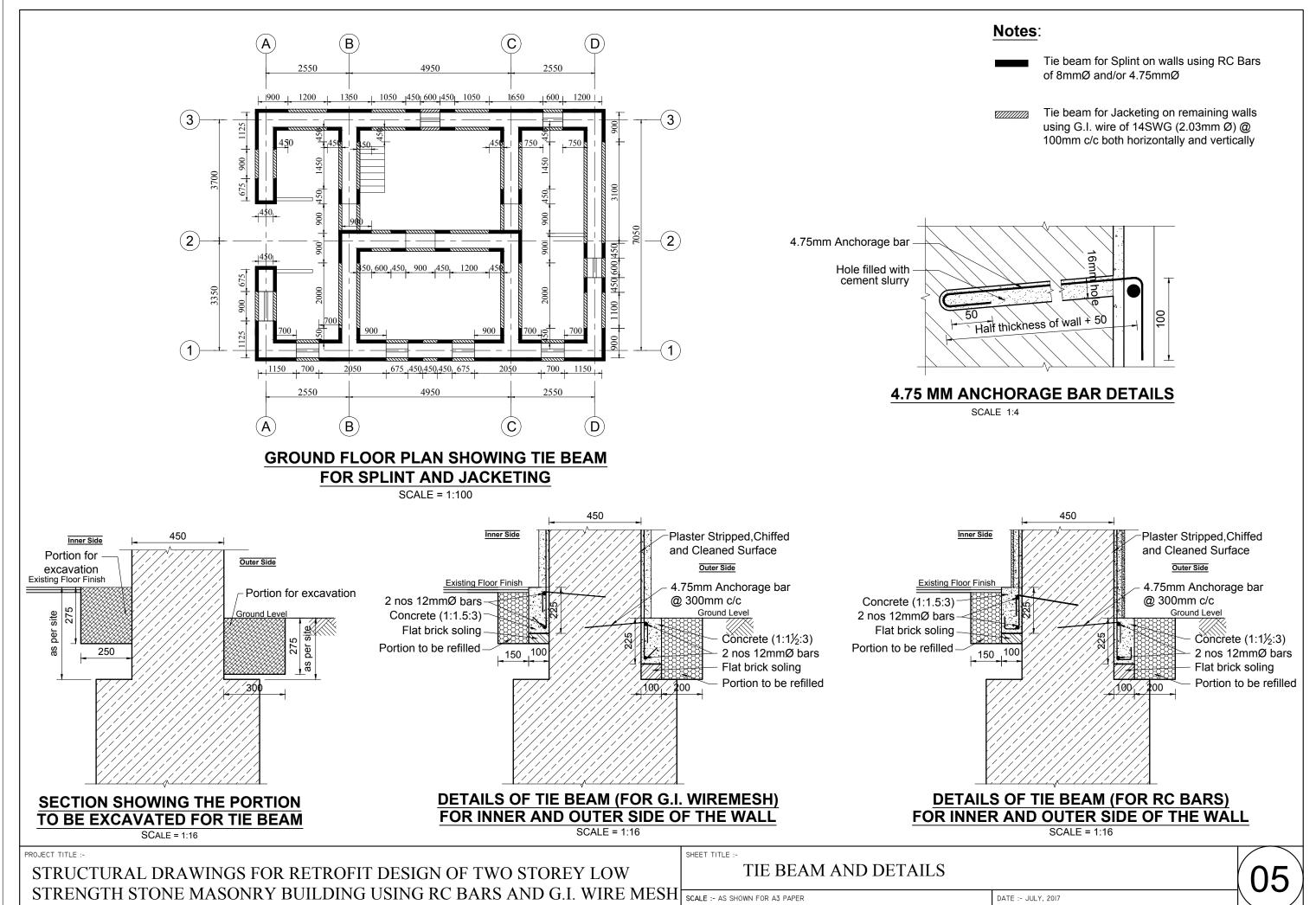


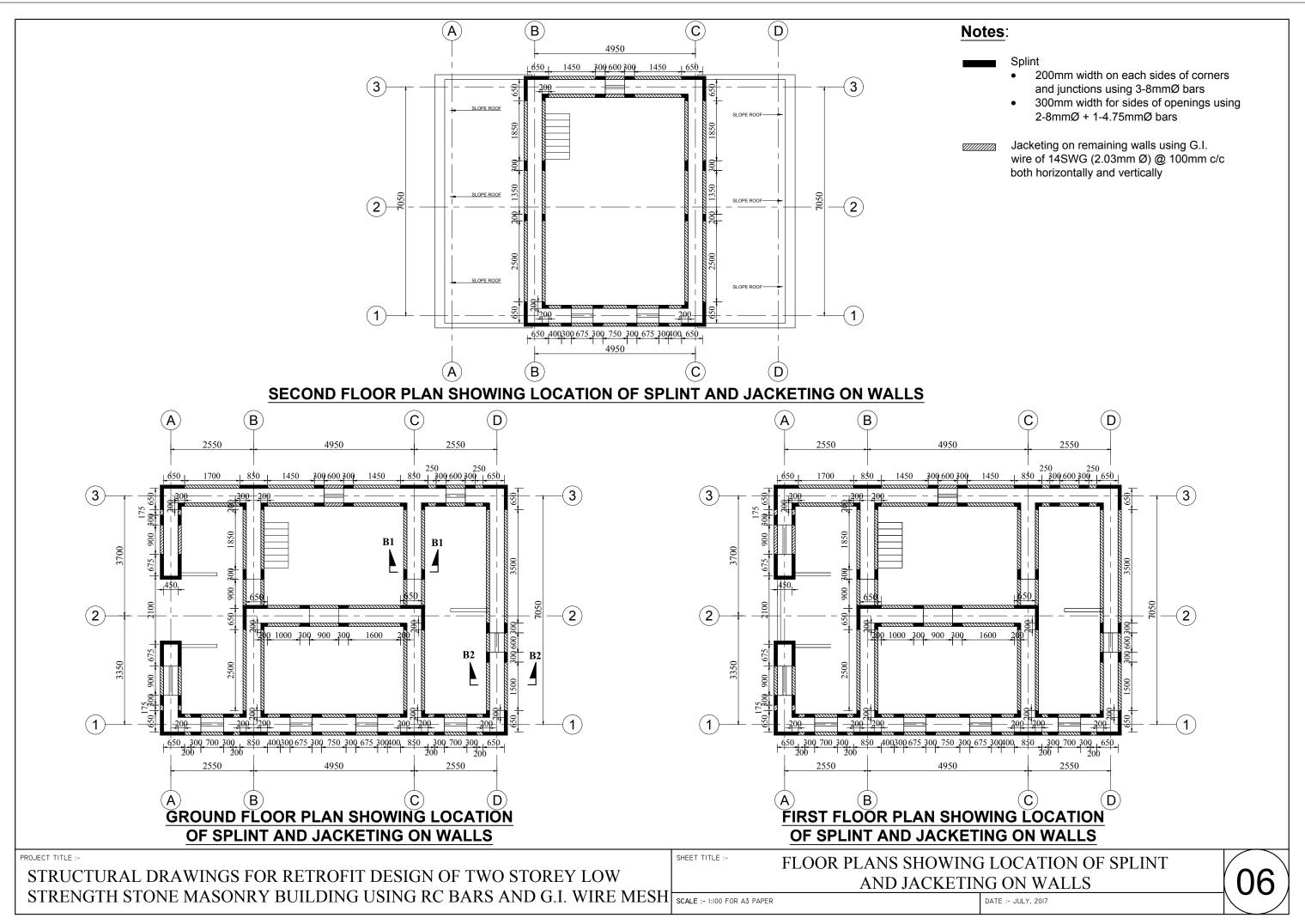
_		
	ELEVATION-1	ELEVATION-3
	Light weight Gable wall	Light weight Gable wa
	ELEVATION-2	ELEVATION-4

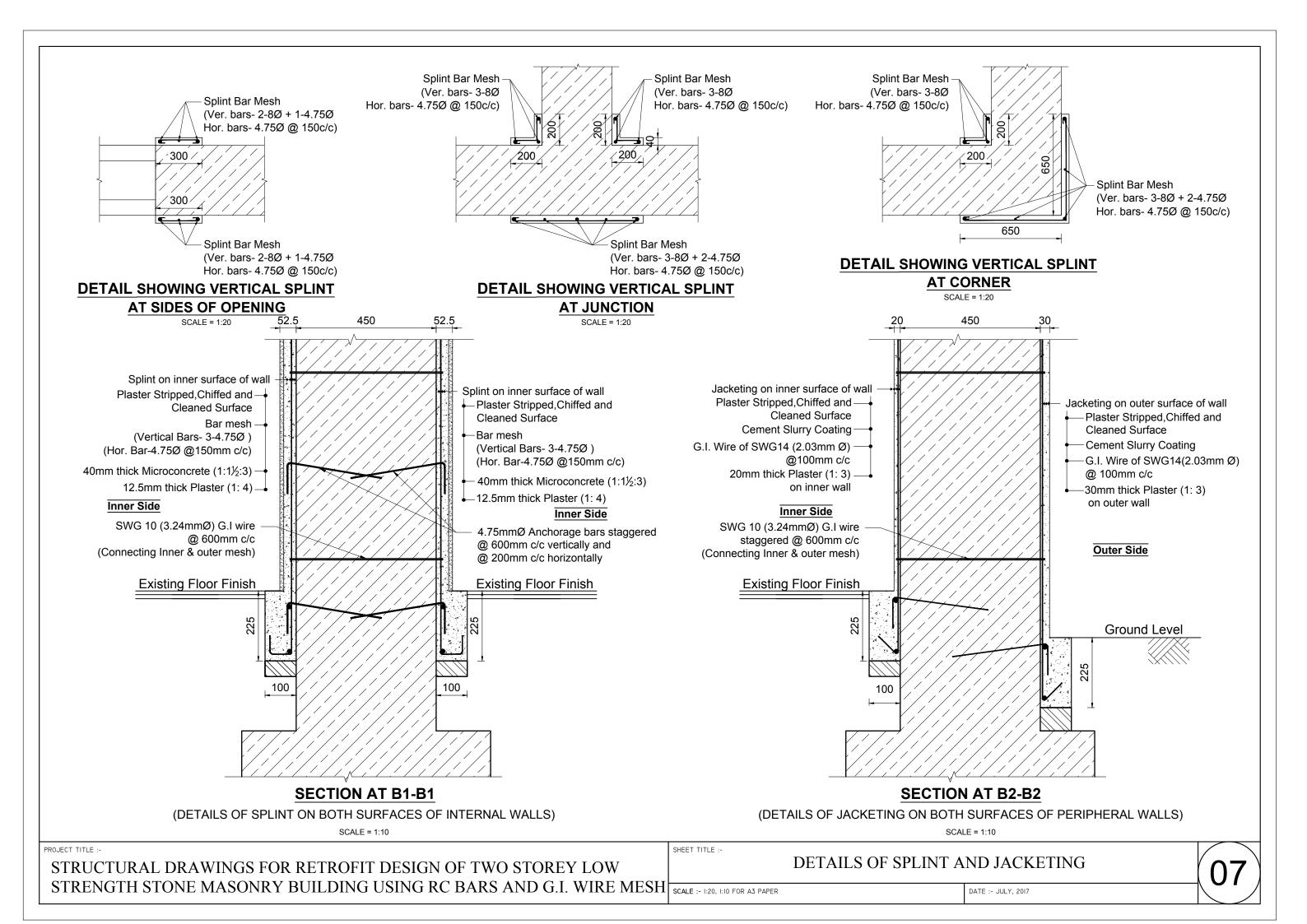
LDING

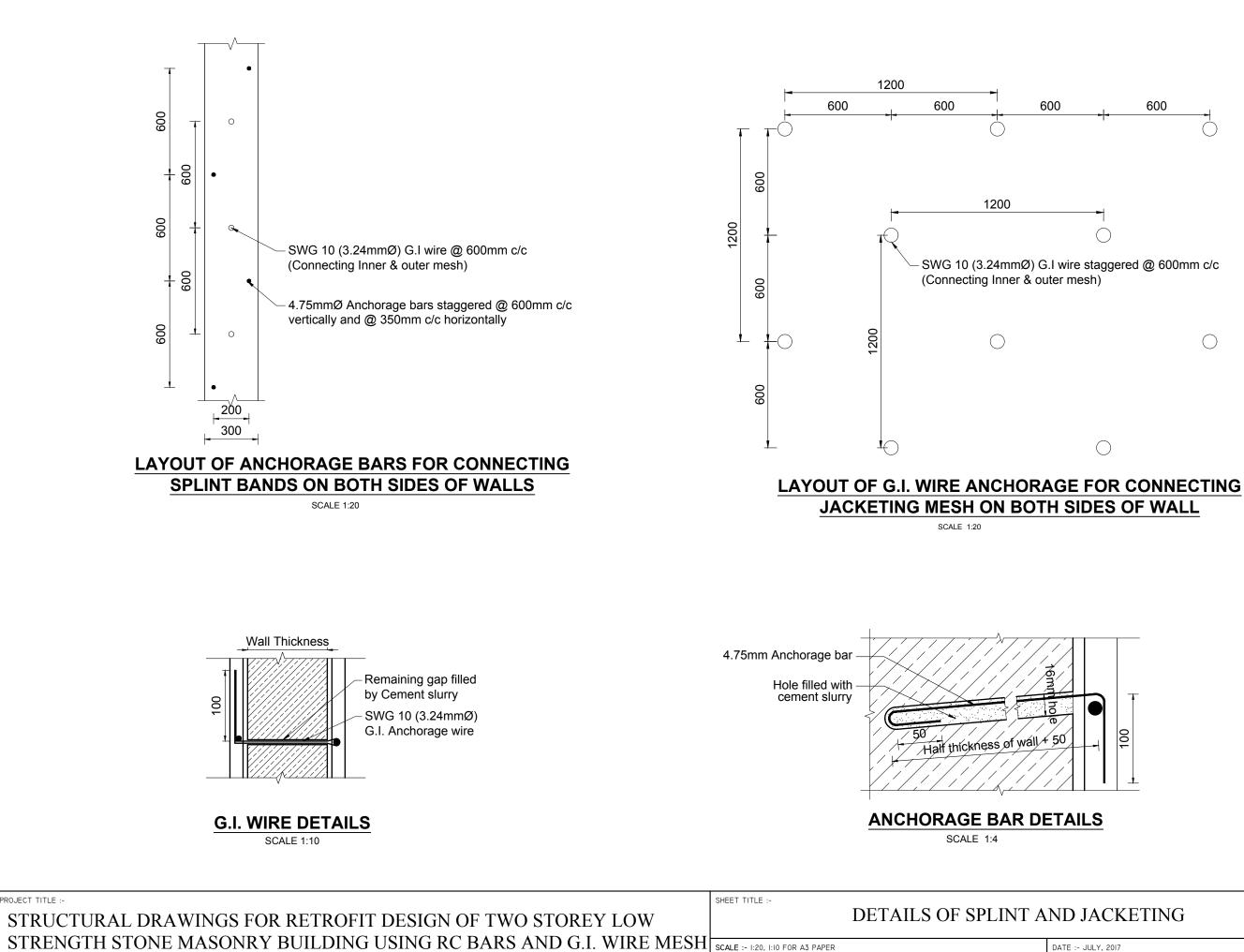
03

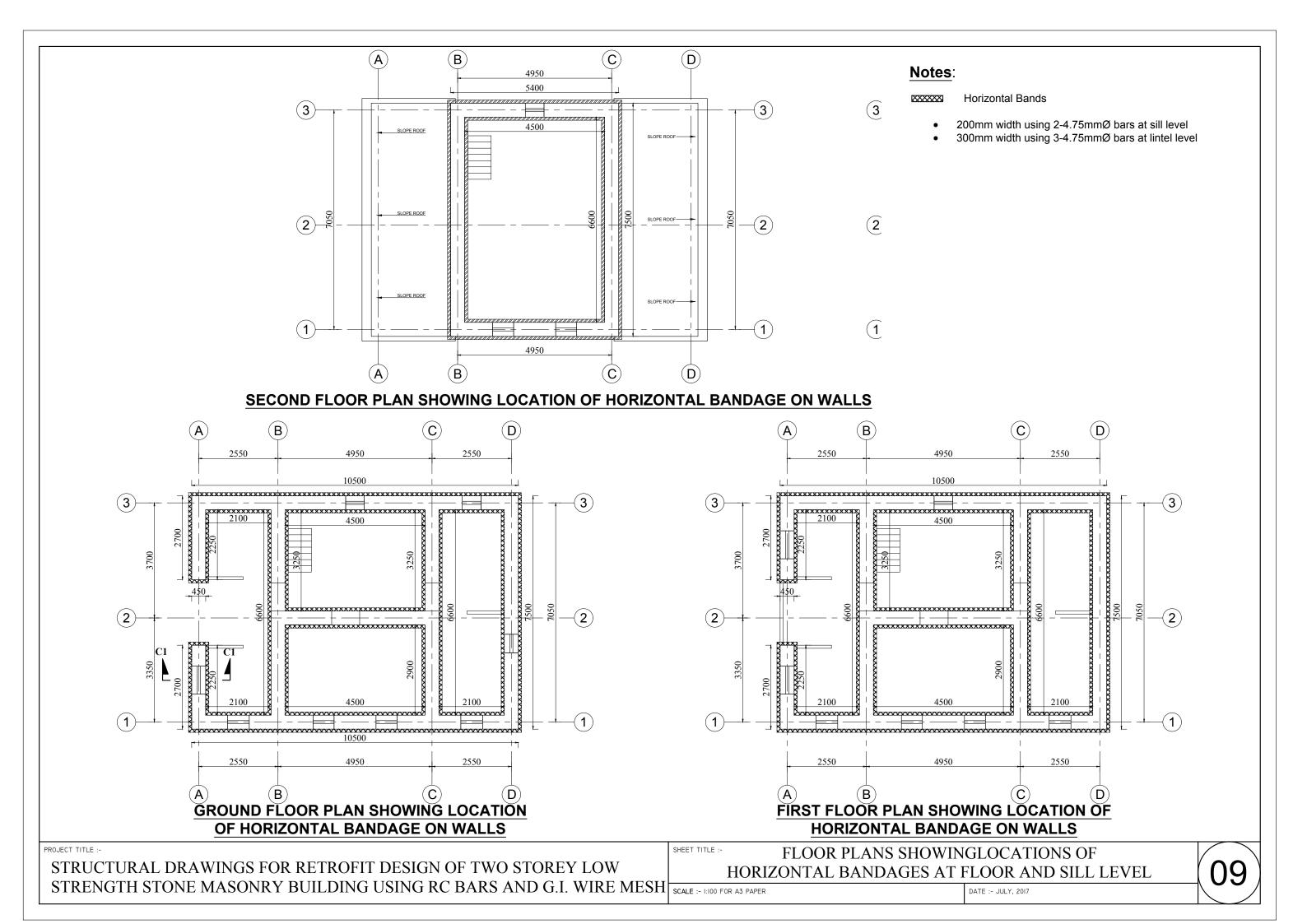


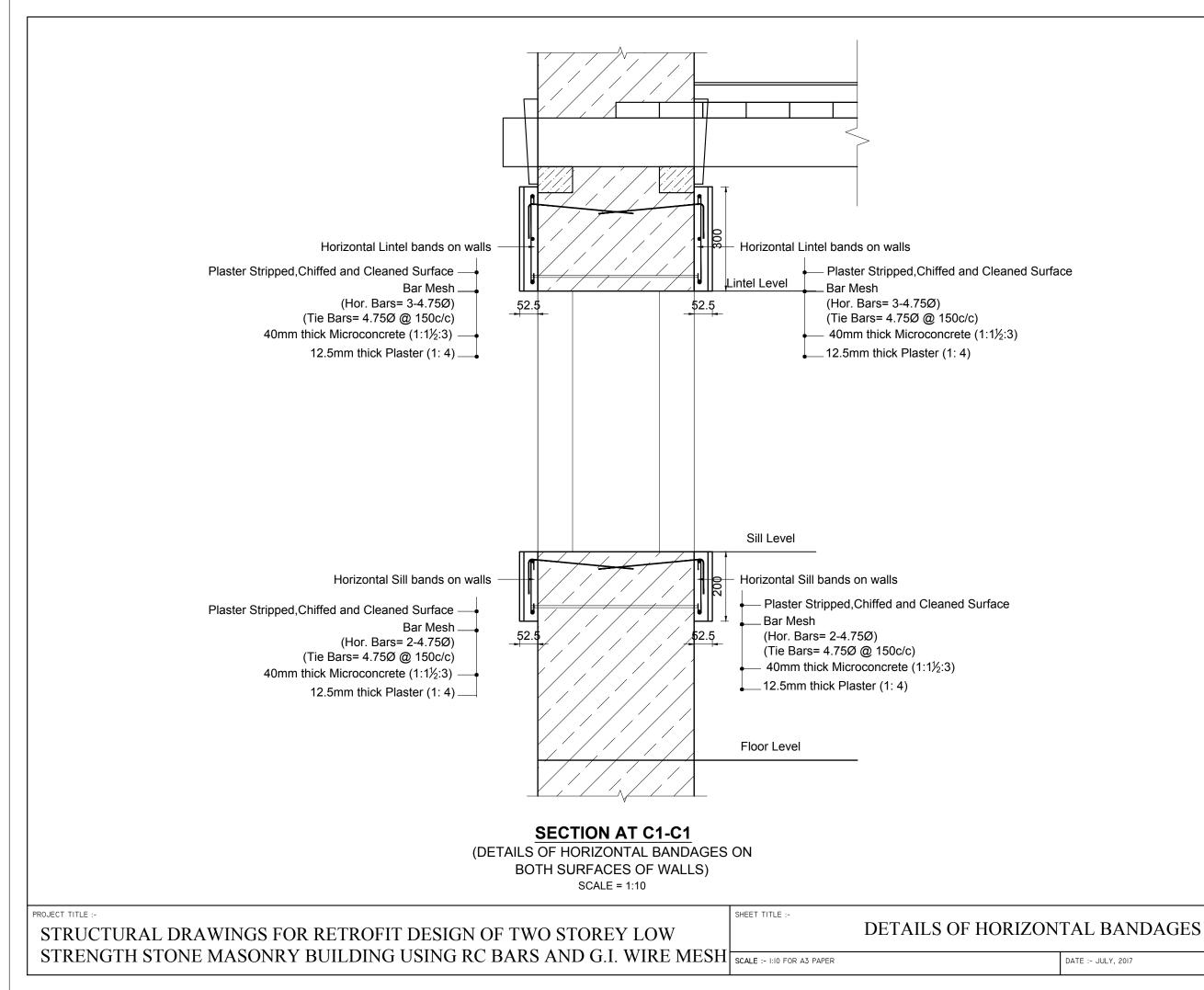


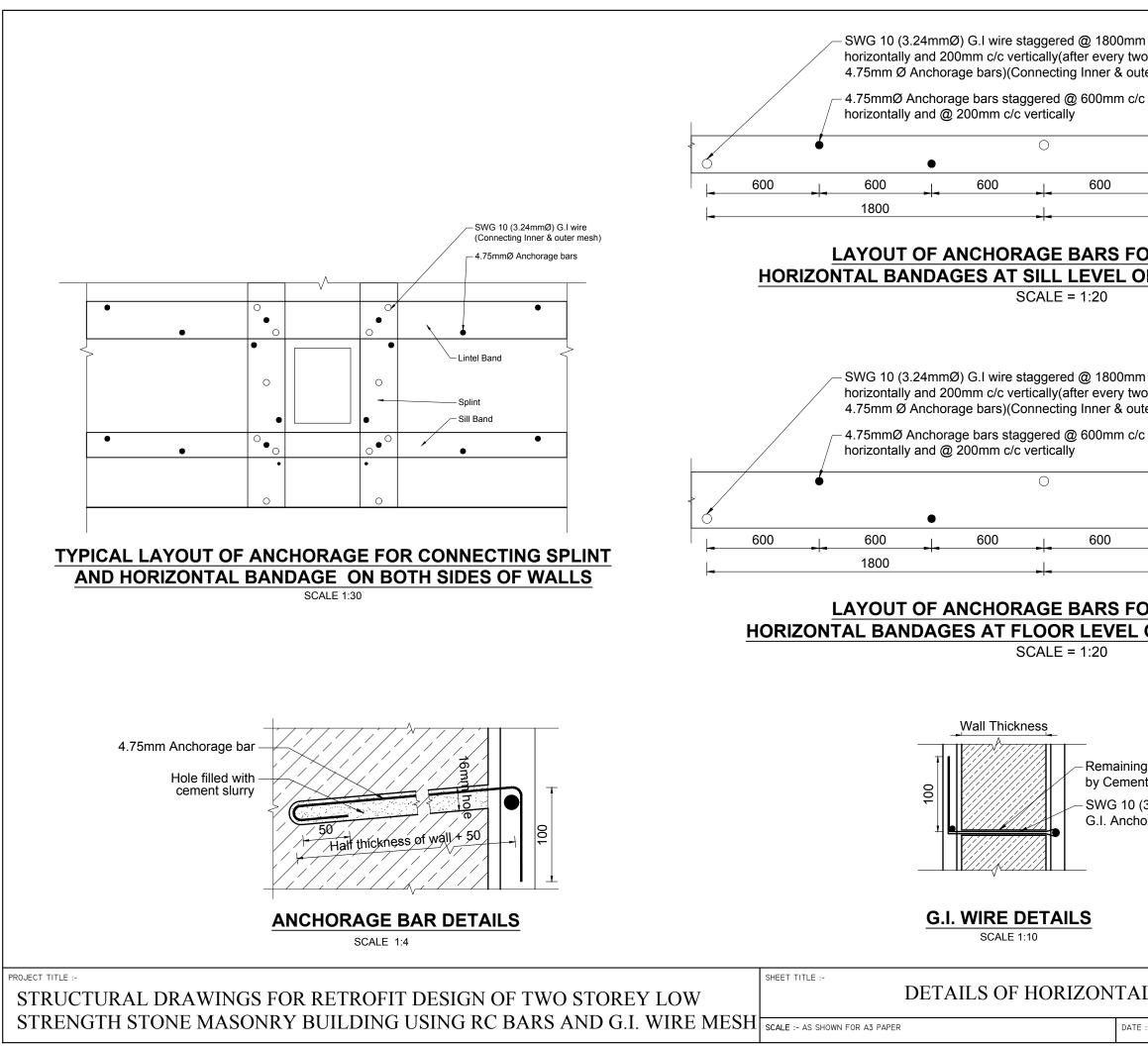




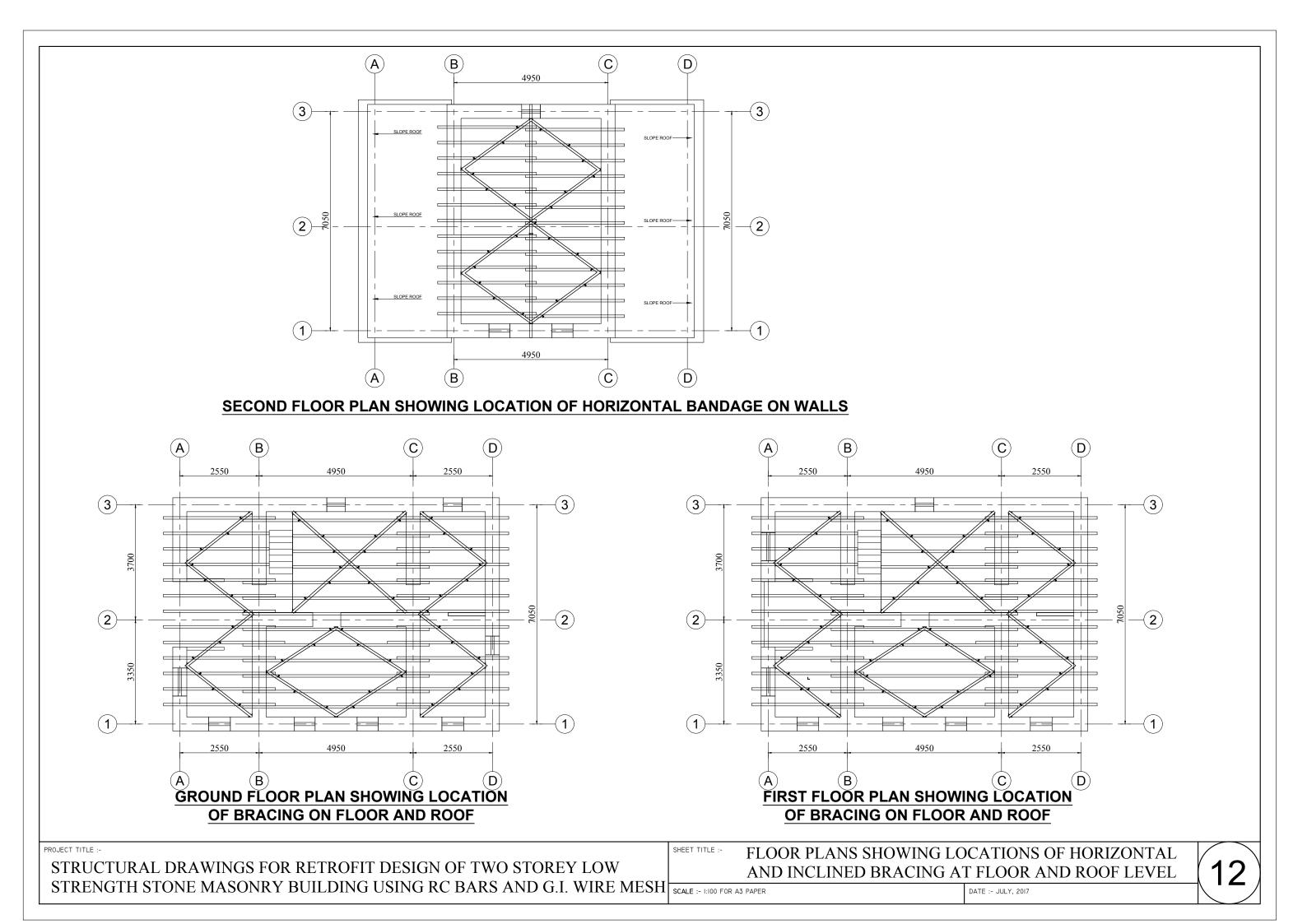


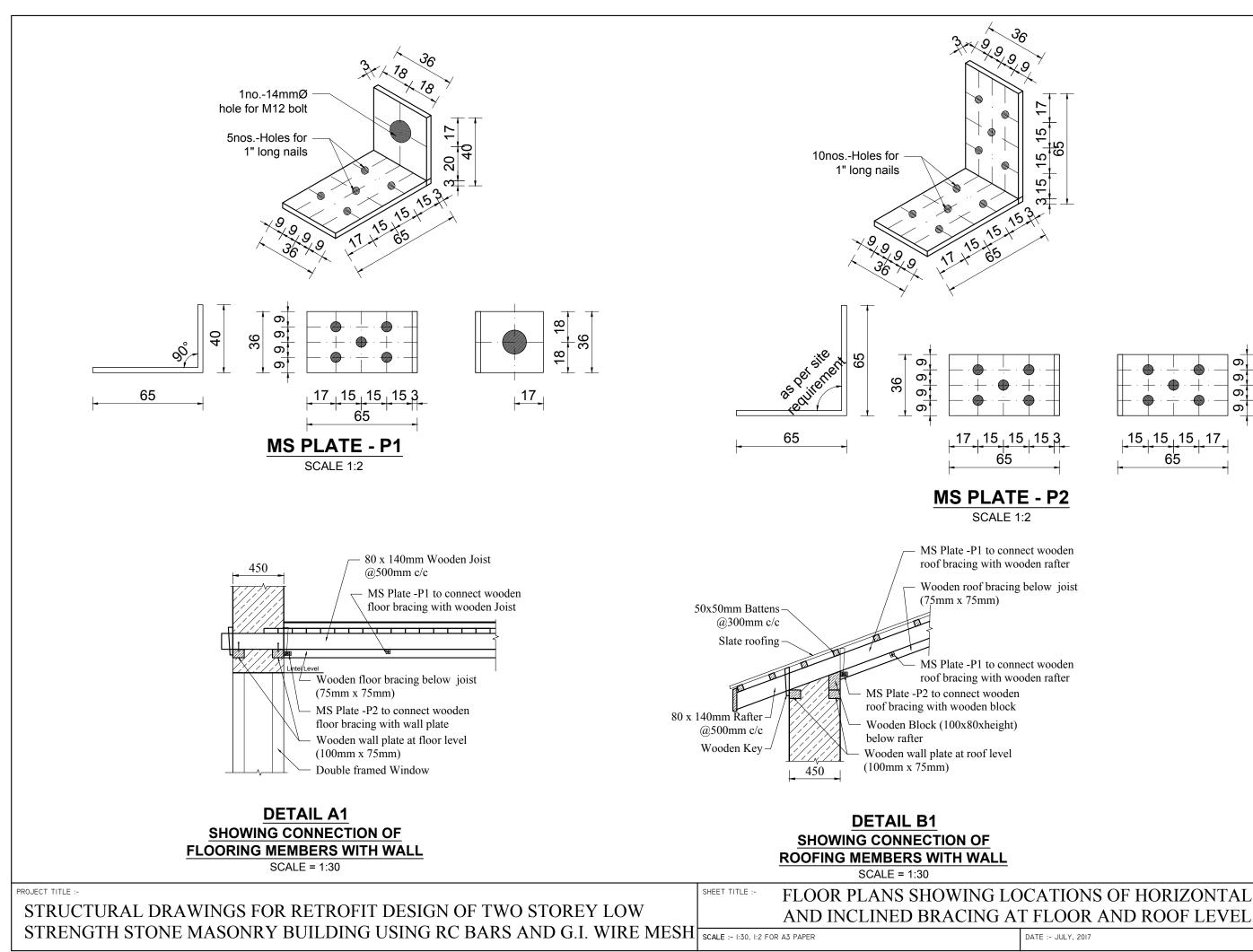






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brage wire									
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:- JULY, 2017		$\overline{\ }$							



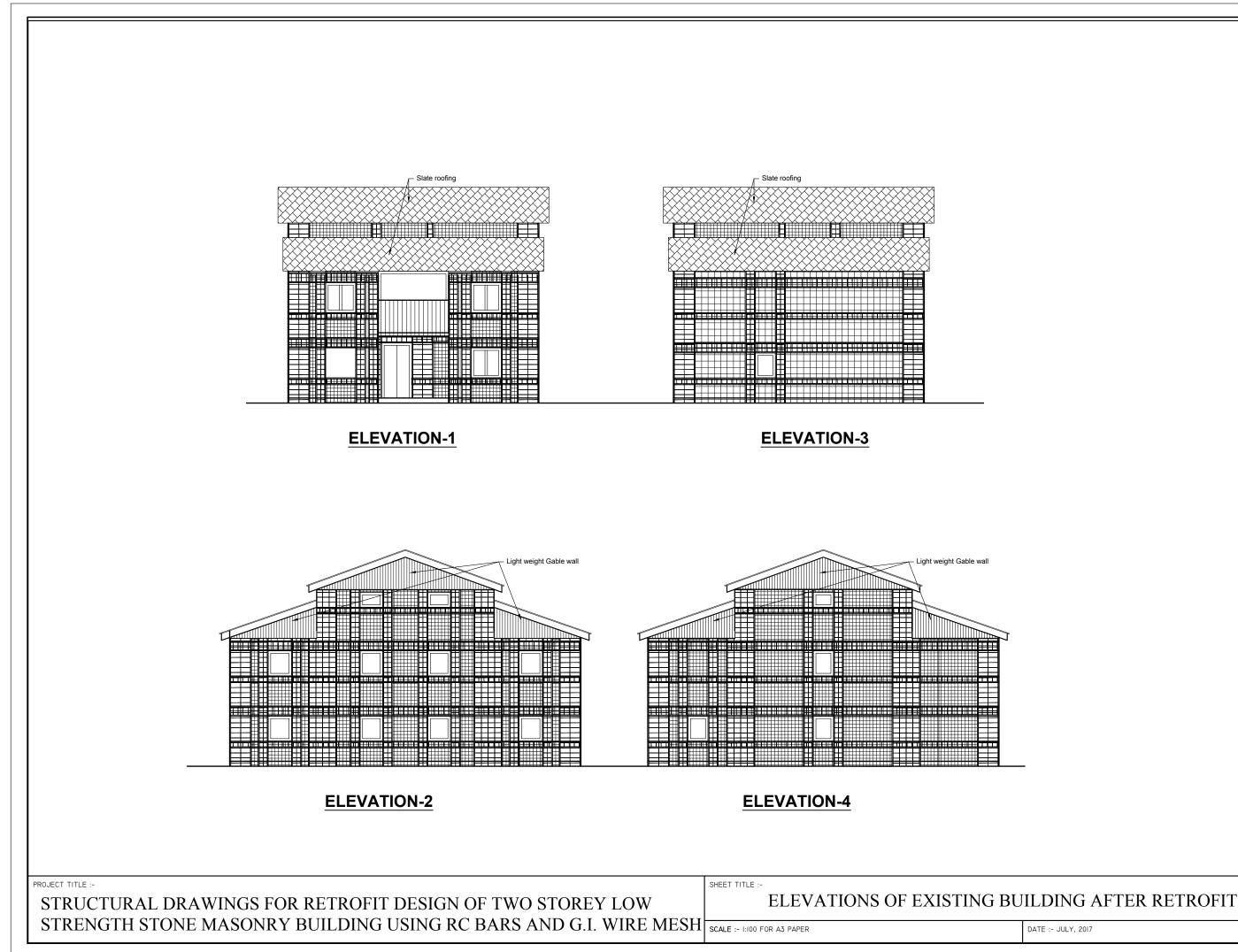


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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON TWO STOREY STONE MASONRY BUILDING IN MUD USING G.I. WIREMESH

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING G.I. WIRE MESHING

SHEET TITLE :-

SCALE :- N/A

COVER PAGE

DA



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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING G.I. WIRE MESHING

4 Elev

<u>10500</u> 4950

(B)

 (\mathbf{A})

2550

 (\mathbf{C})

SCALE :- 1:100 FOR A3 PAPER

SHEET TITLE :-

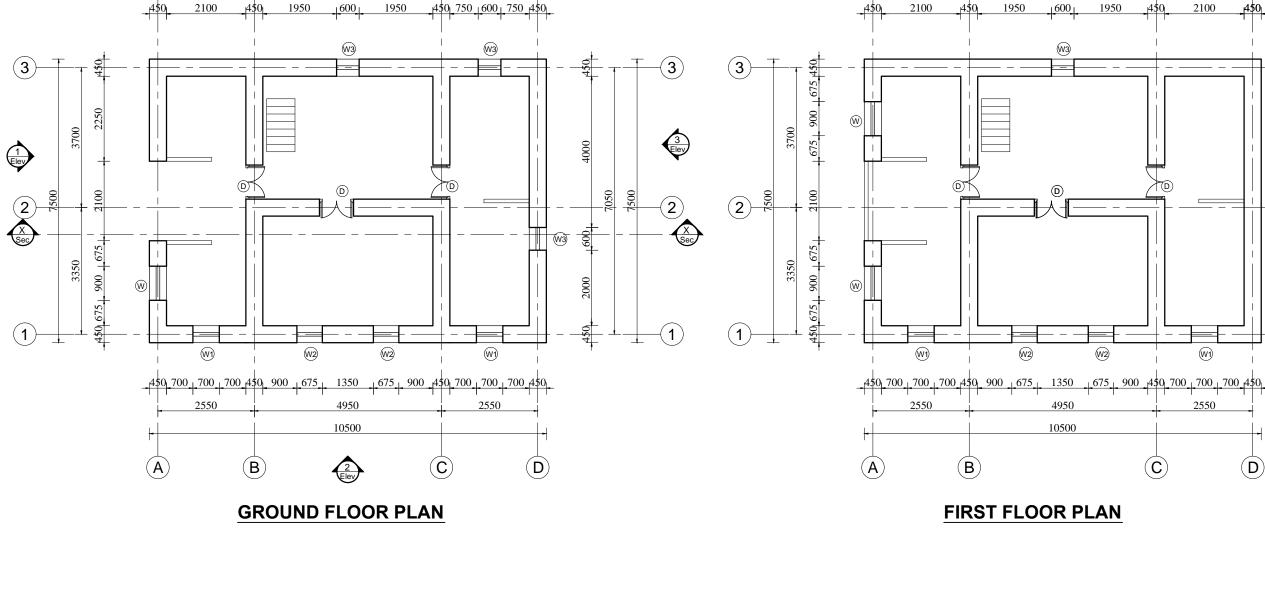
FLOOR PLANS OF EXISTING BUILDING

(B)

2550

(A)

PROJECT TITLE :-



 (D)

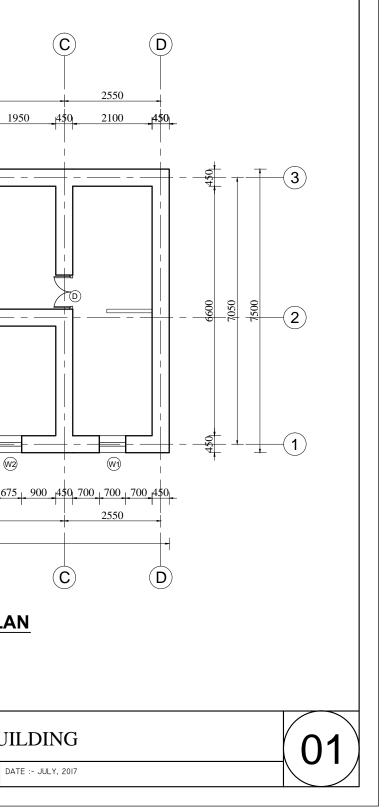
2550

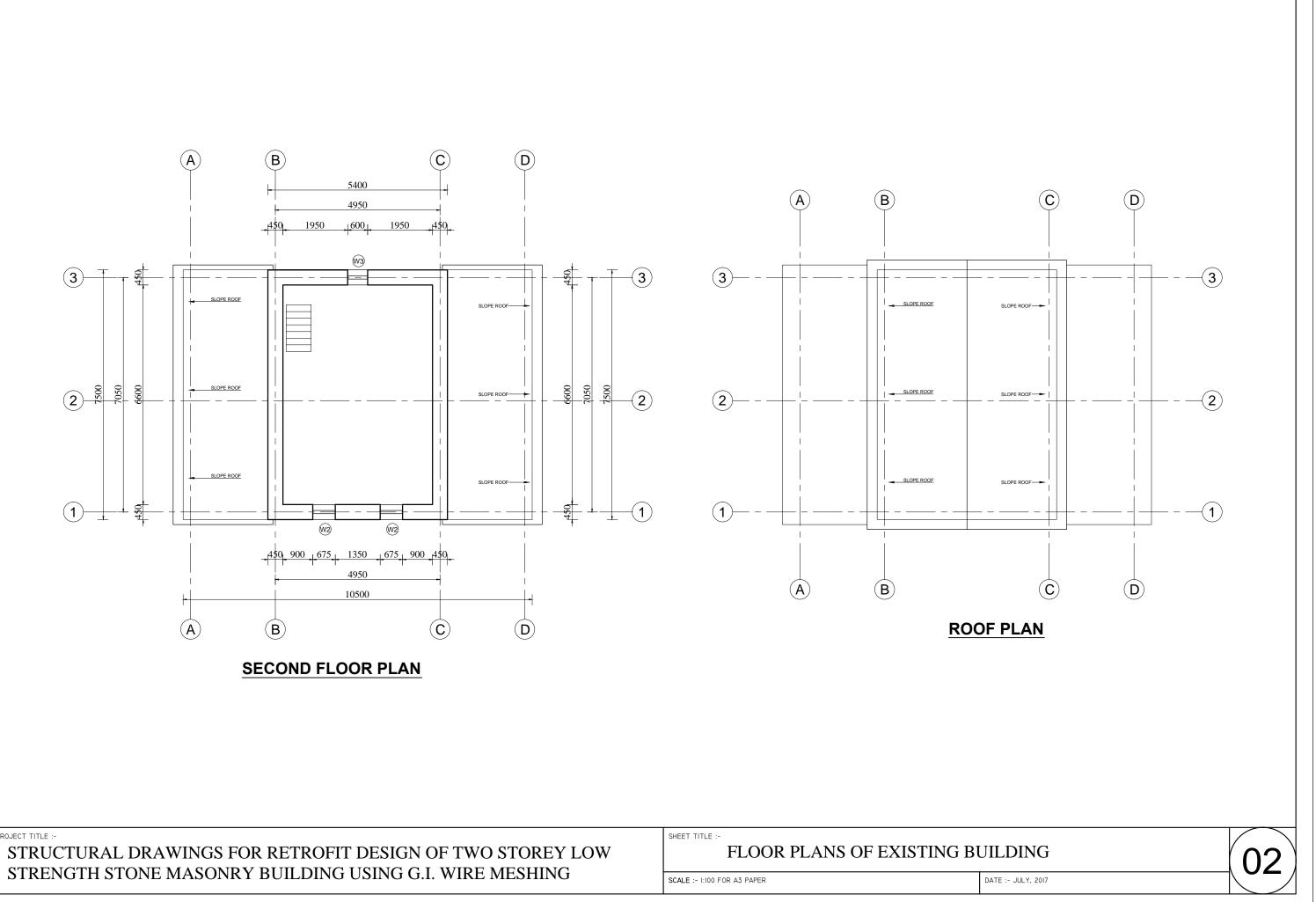
OPEN S.N. 1. 2. 3. 4. 5.

4950

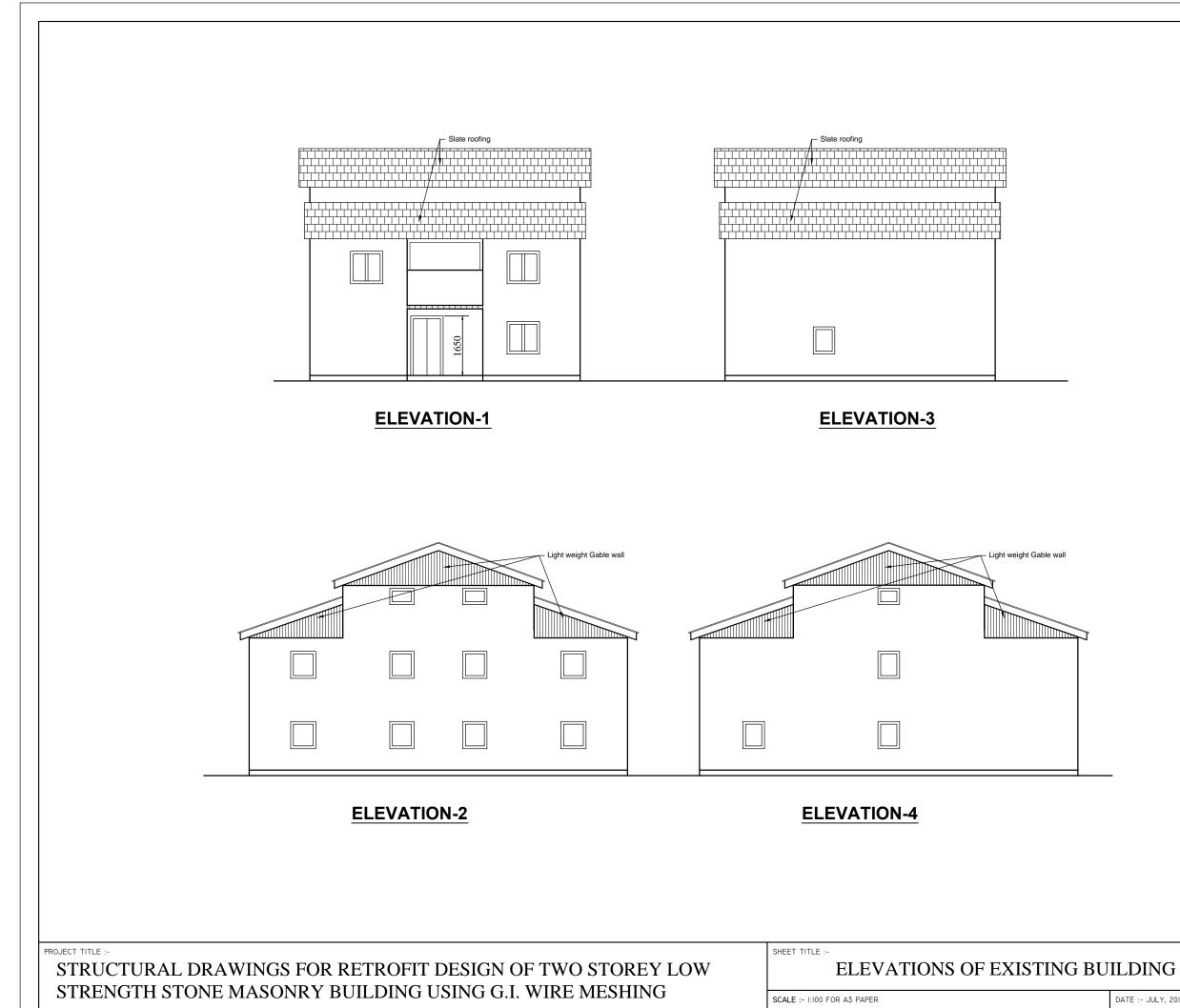
OPENING SCHEDULE

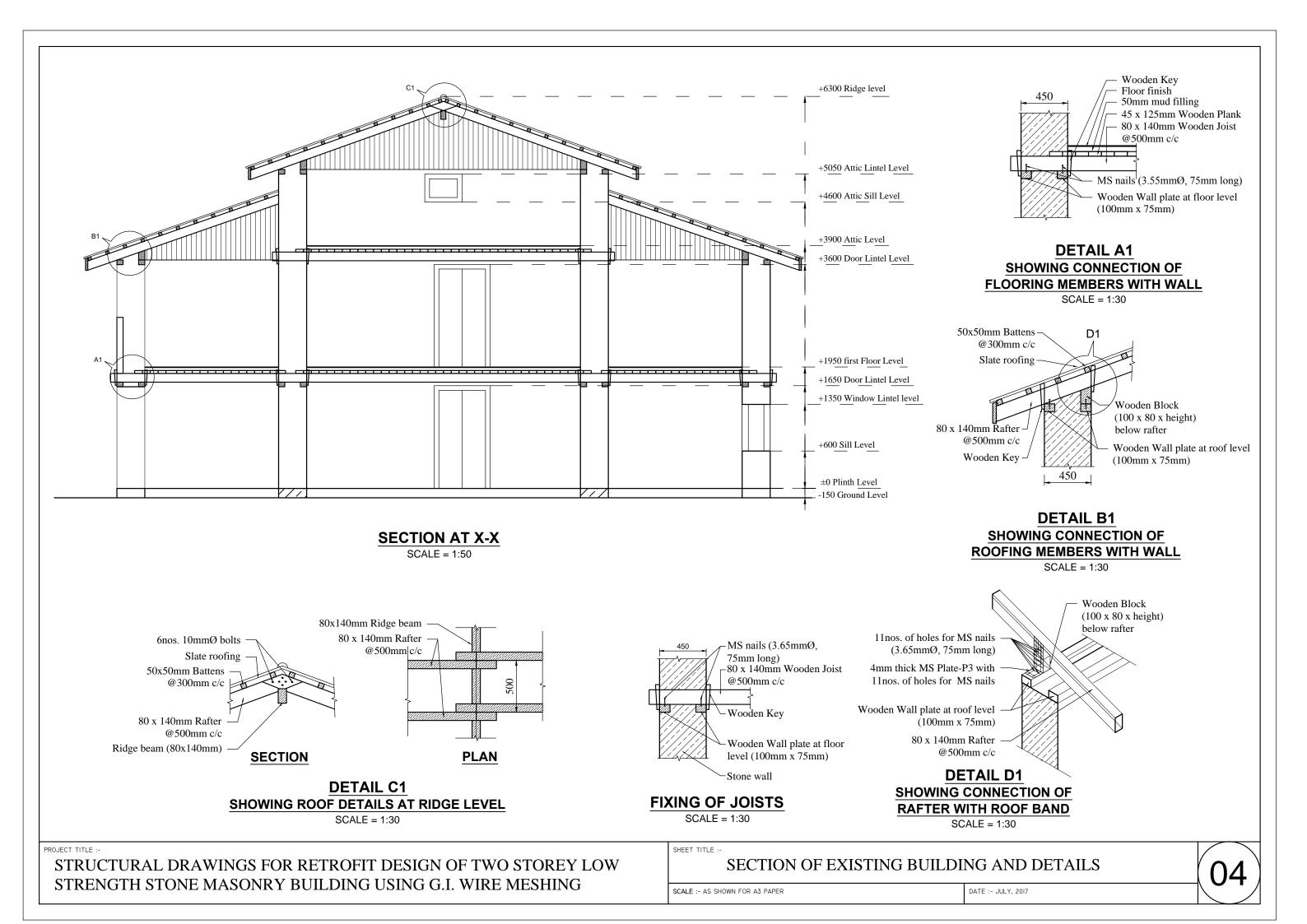
I.	SYMBOL	NOS.	SIZE	SILL HEIGHT
	DOOR-D	6	900 x 1650	
	WINDOW-W	3	900 x 900	600
	WINDOW-W1	4	700 x 750	600
	WINDOW-W2	6	675 x 750	600
	WINDOW-W3	6	600 x 750	600

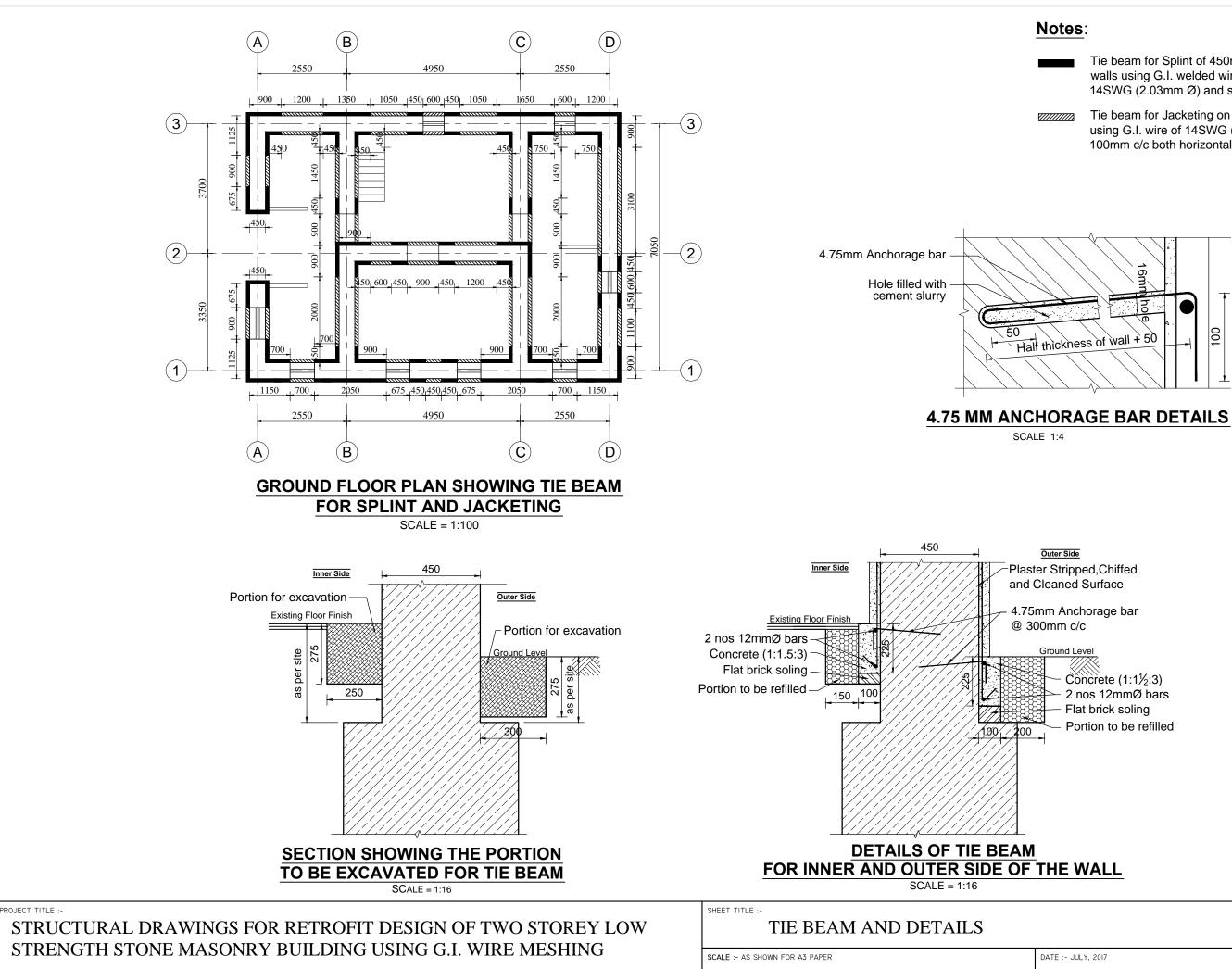




PROJECT TITLE :-STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW

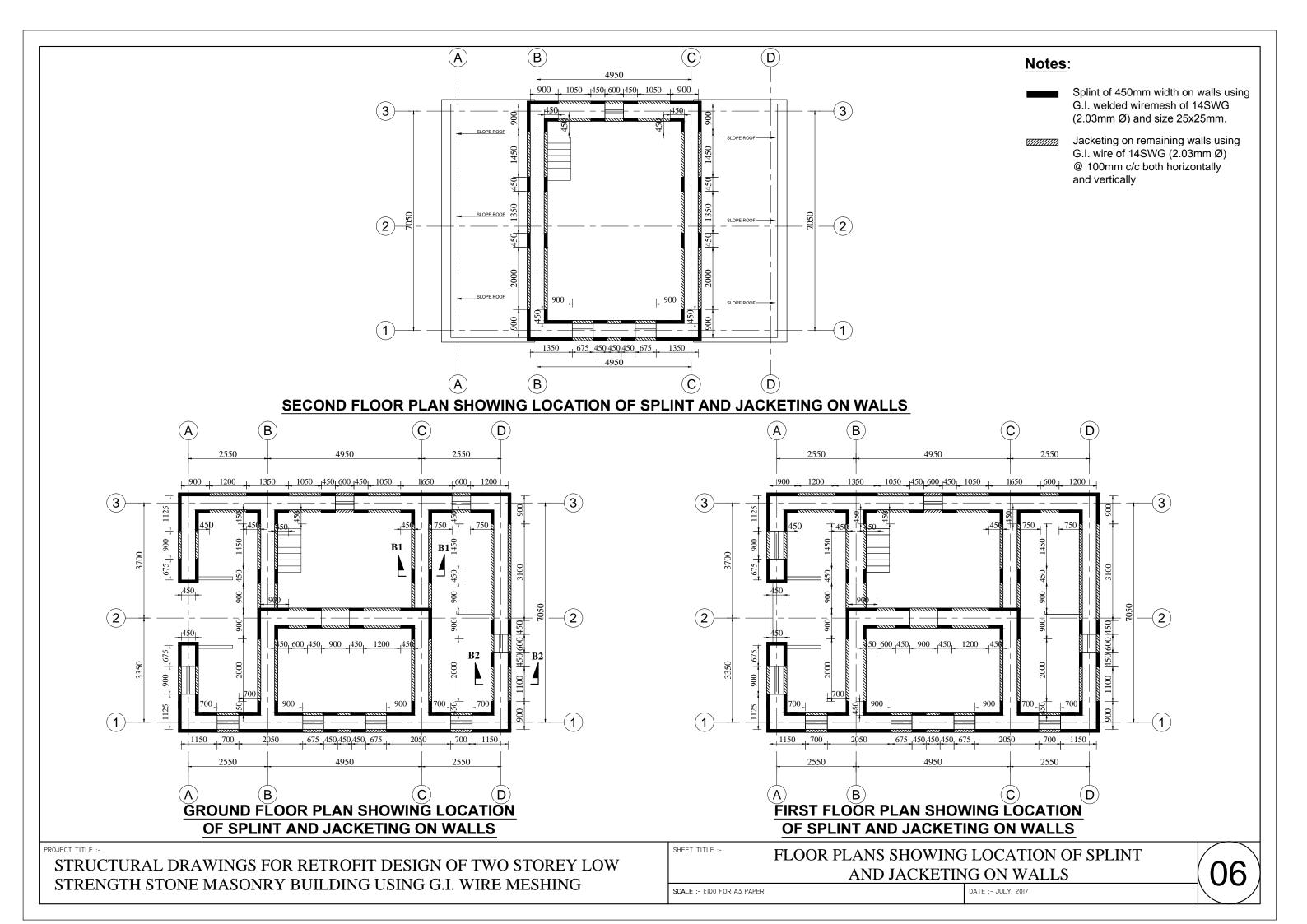


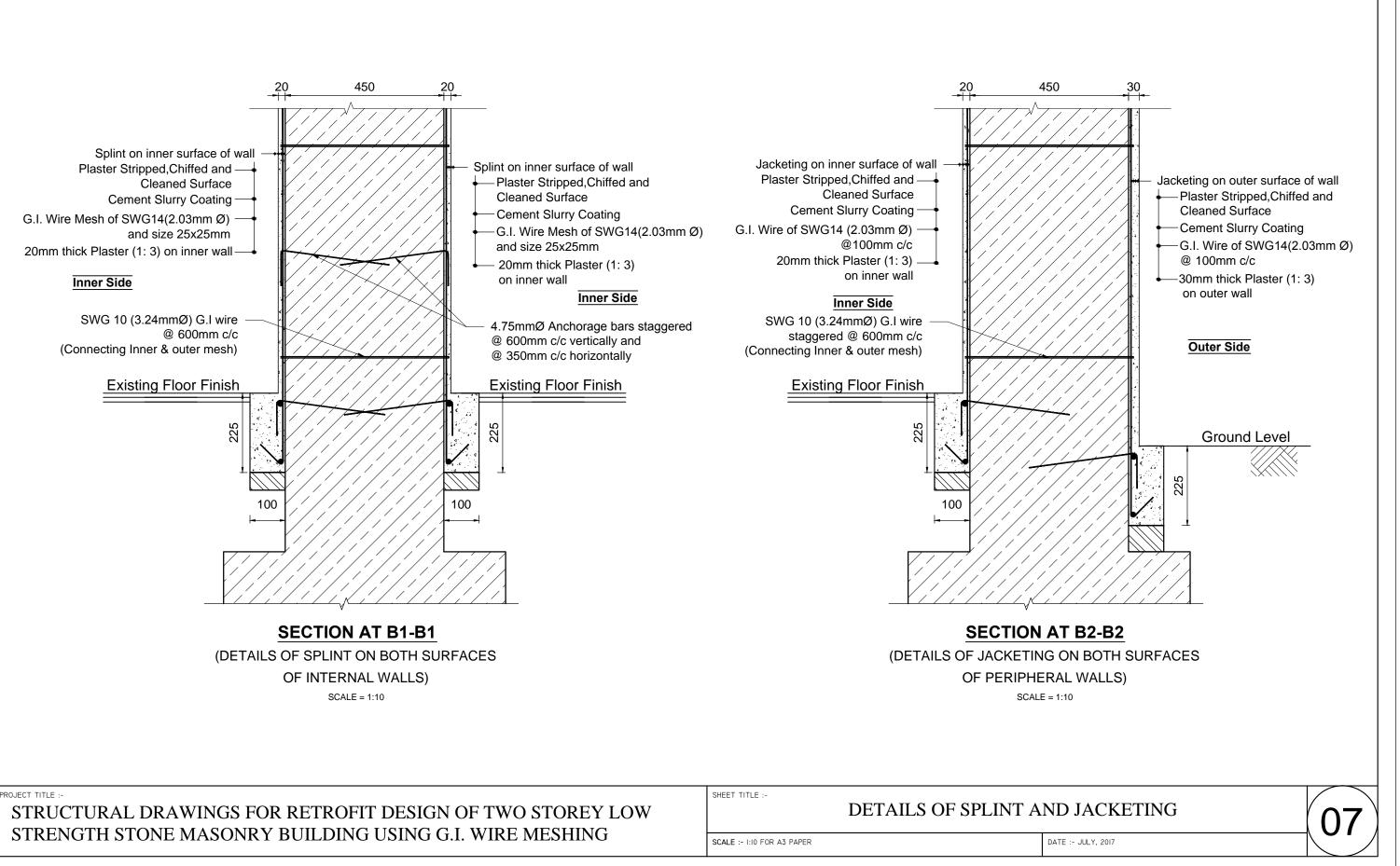


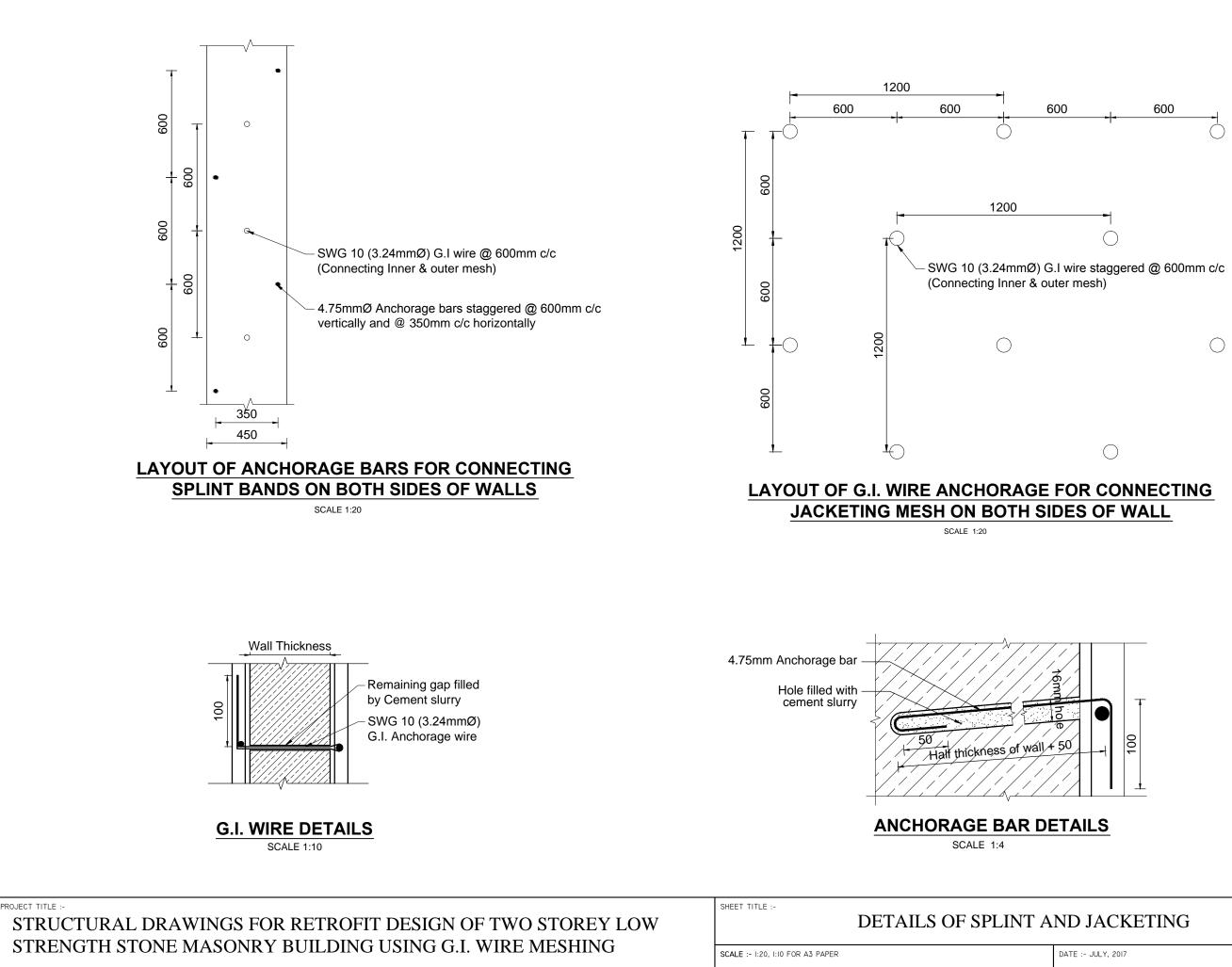


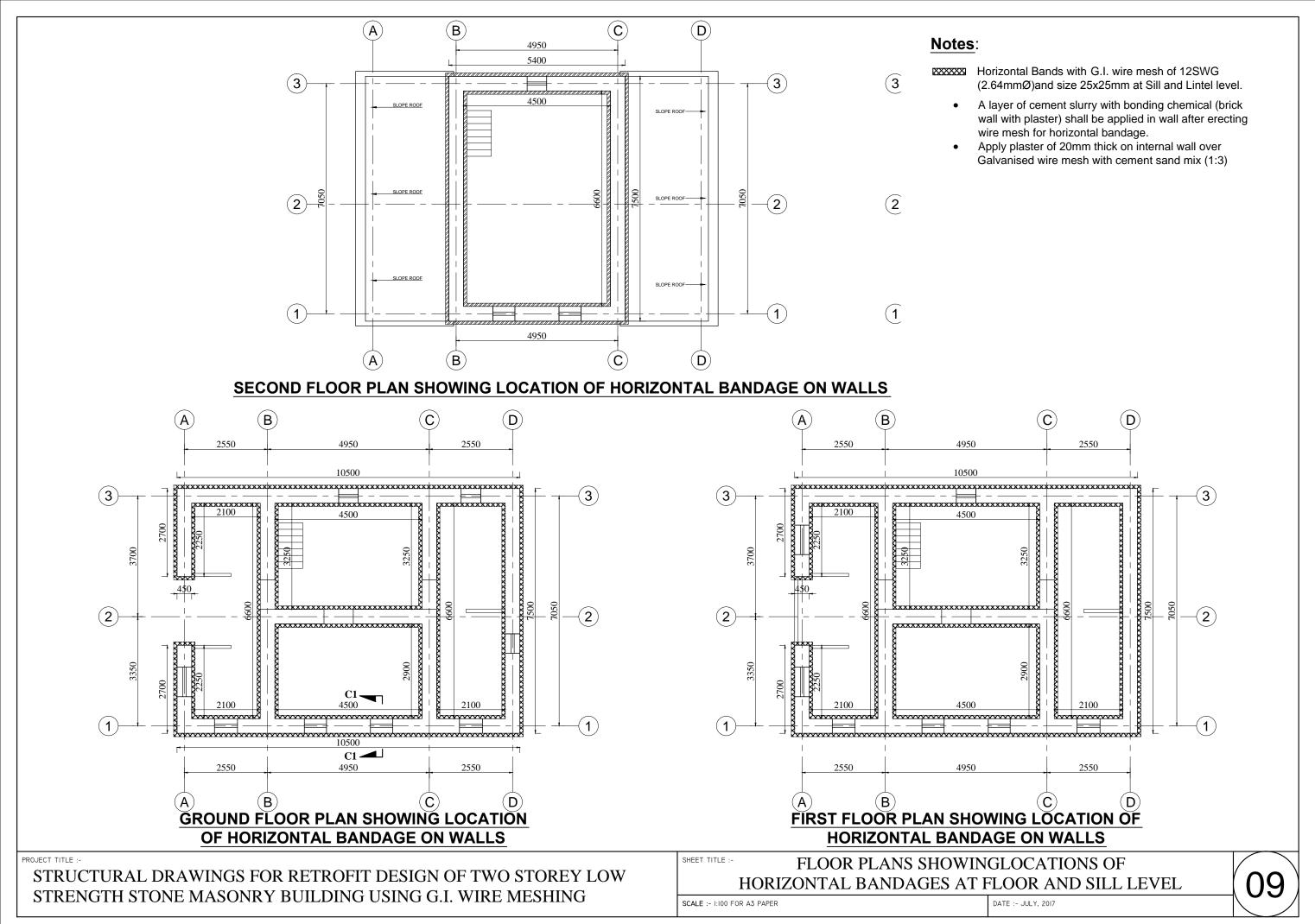
Tie beam for Splint of 450mm width on walls using G.I. welded wiremesh of 14SWG (2.03mm Ø) and size 25x25mm.

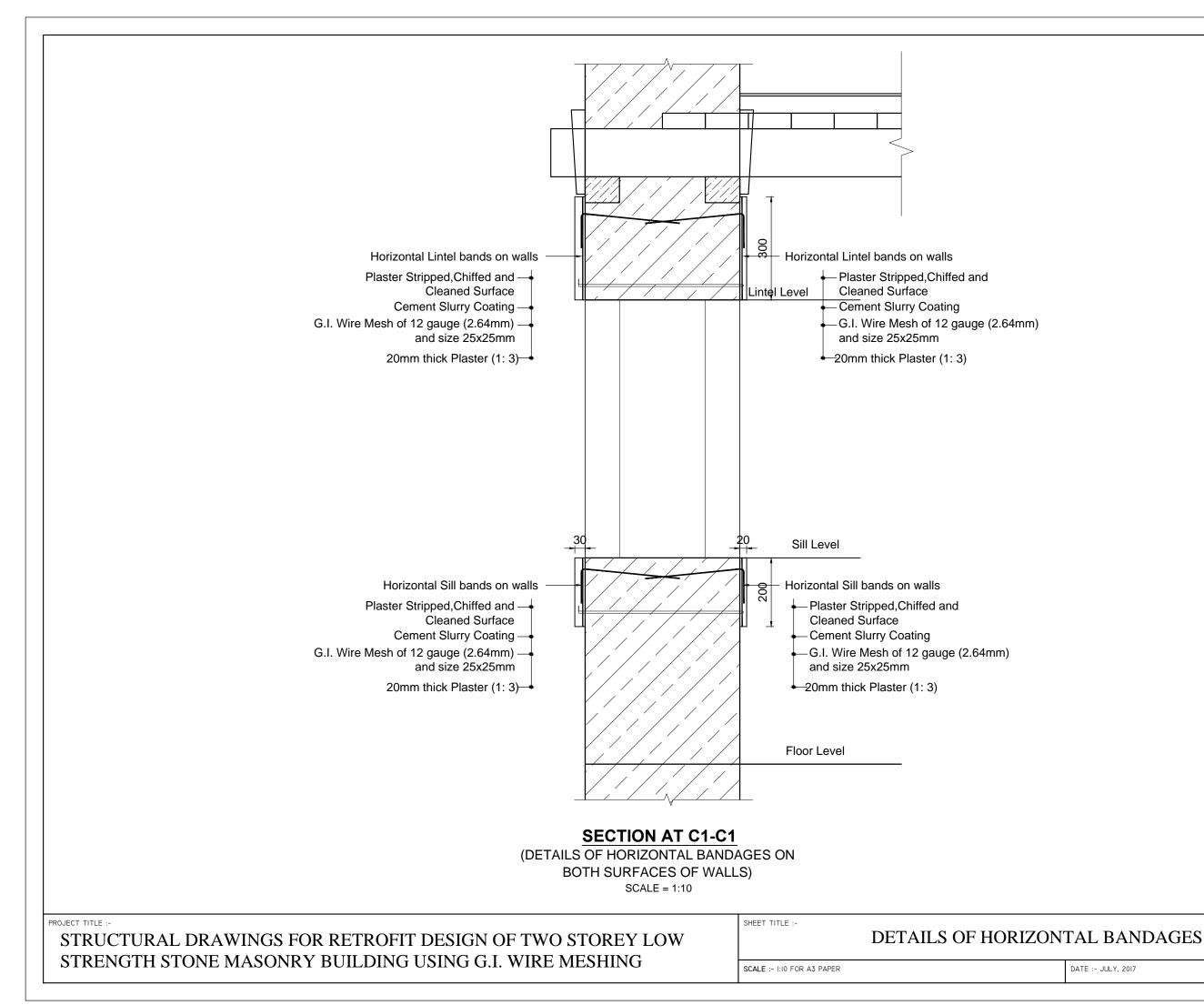
Tie beam for Jacketing on remaining walls using G.I. wire of 14SWG (2.03mm Ø) @ 100mm c/c both horizontally and vertically

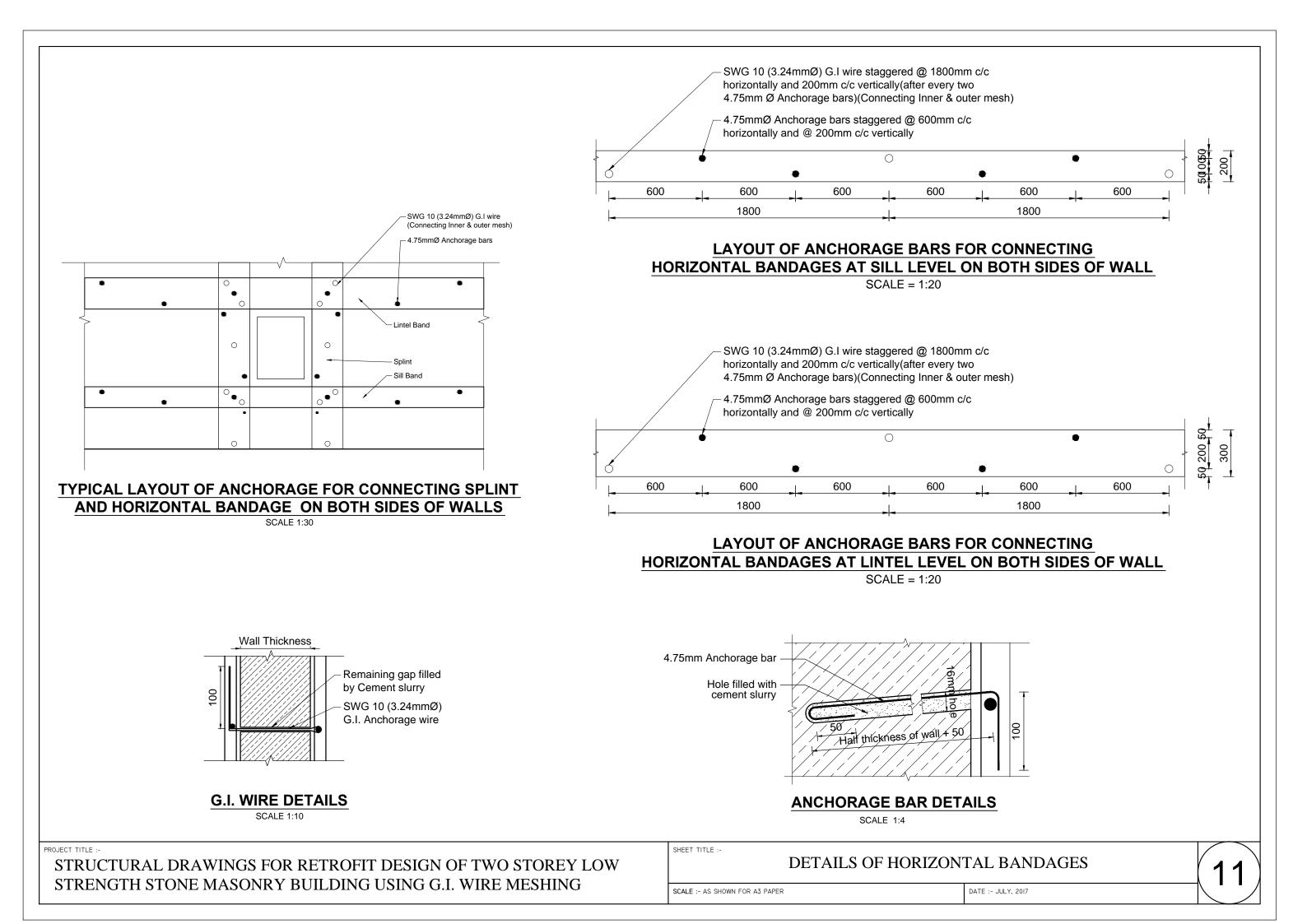


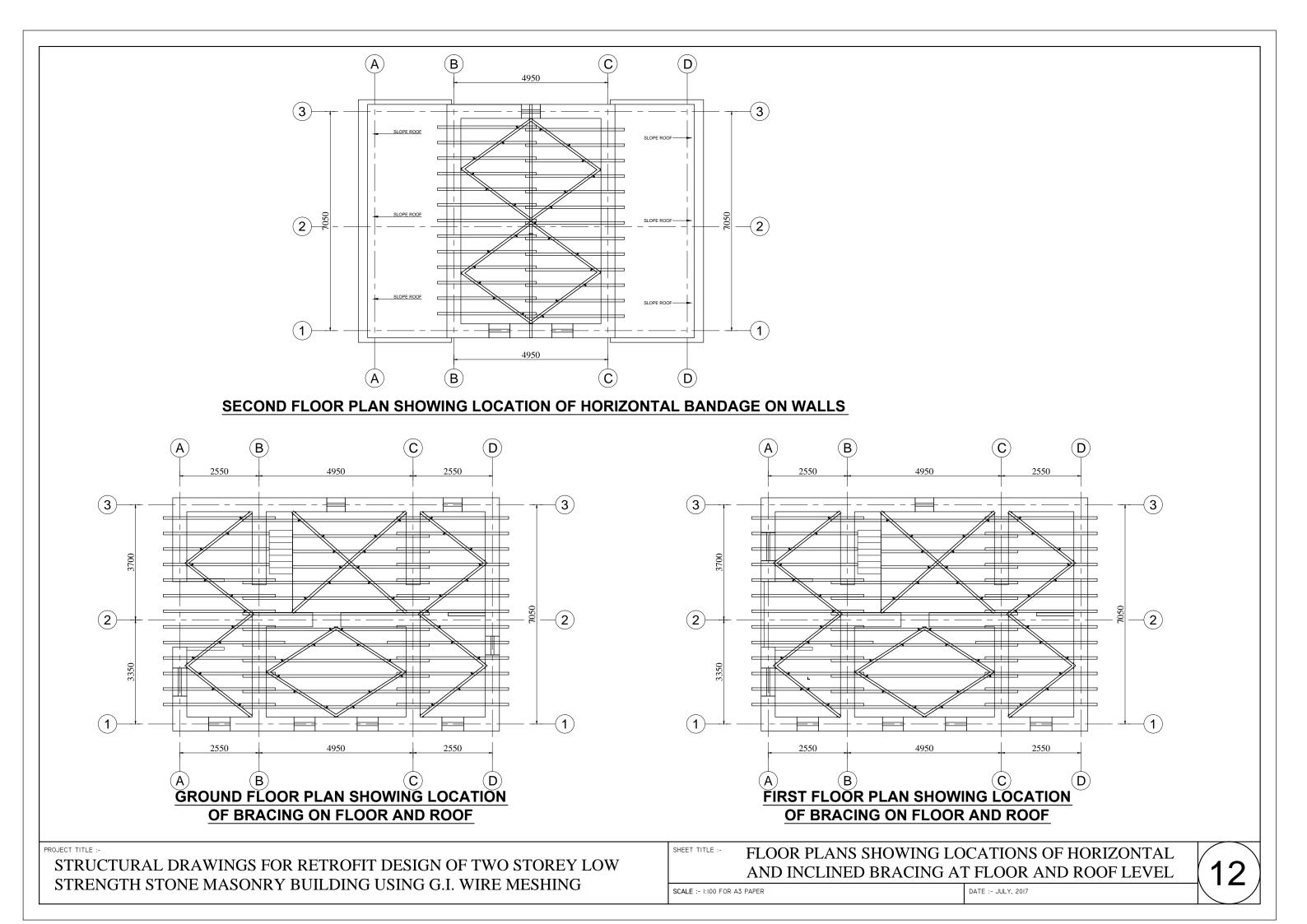


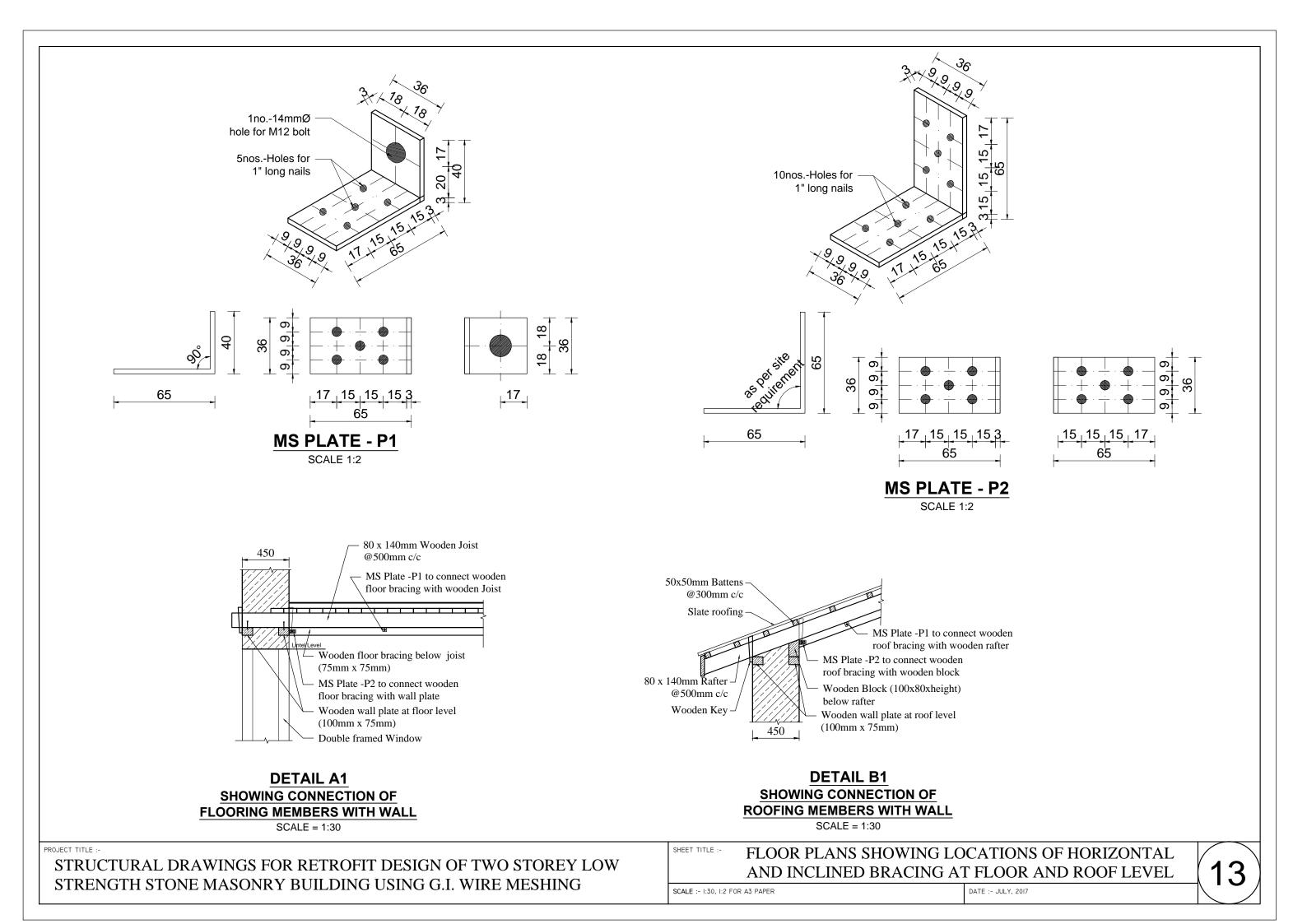


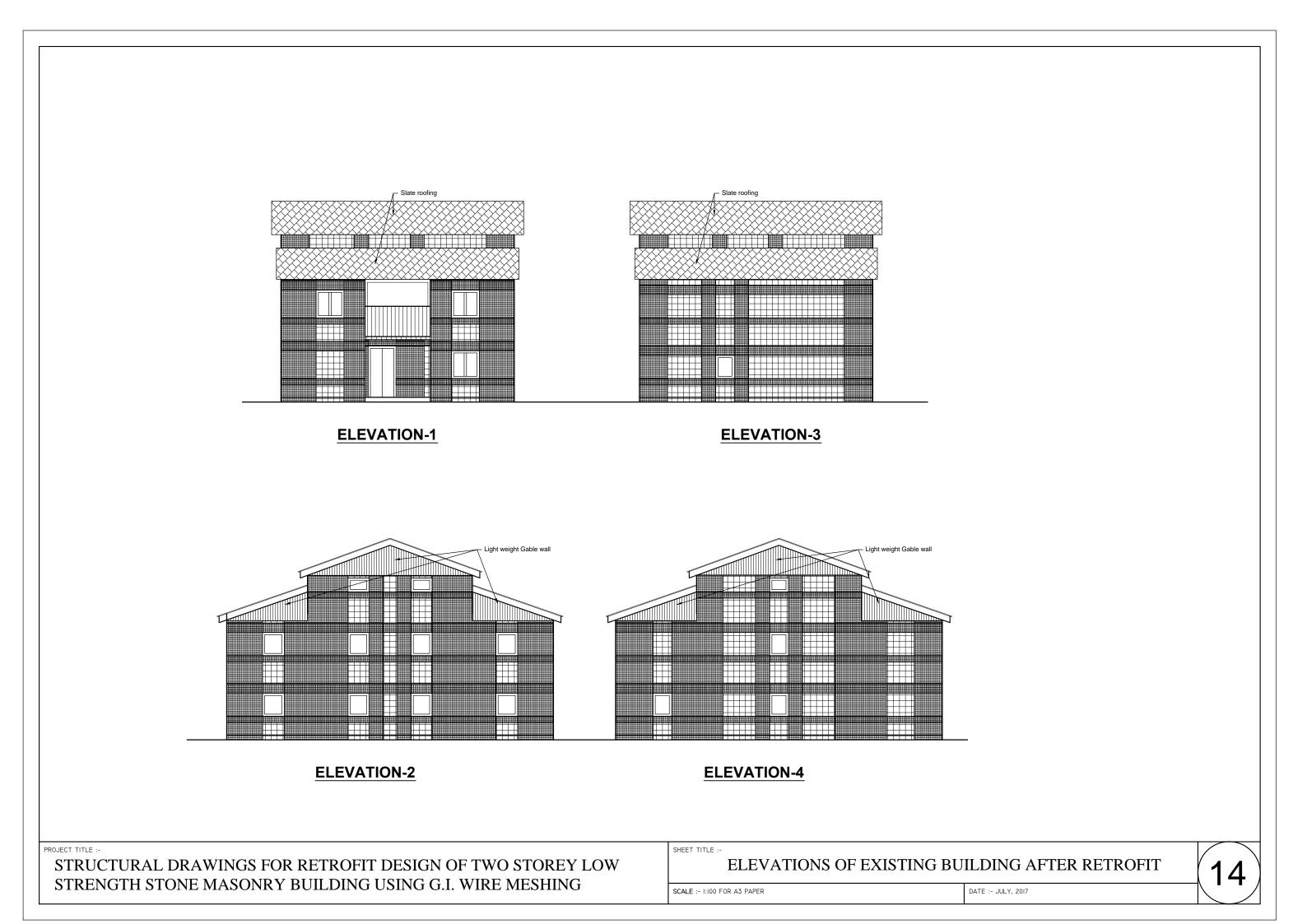












STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON TWO STOREY STONE MASONRY BUILDING IN MUD USING G.I. WIREMESH AND PP BANDS

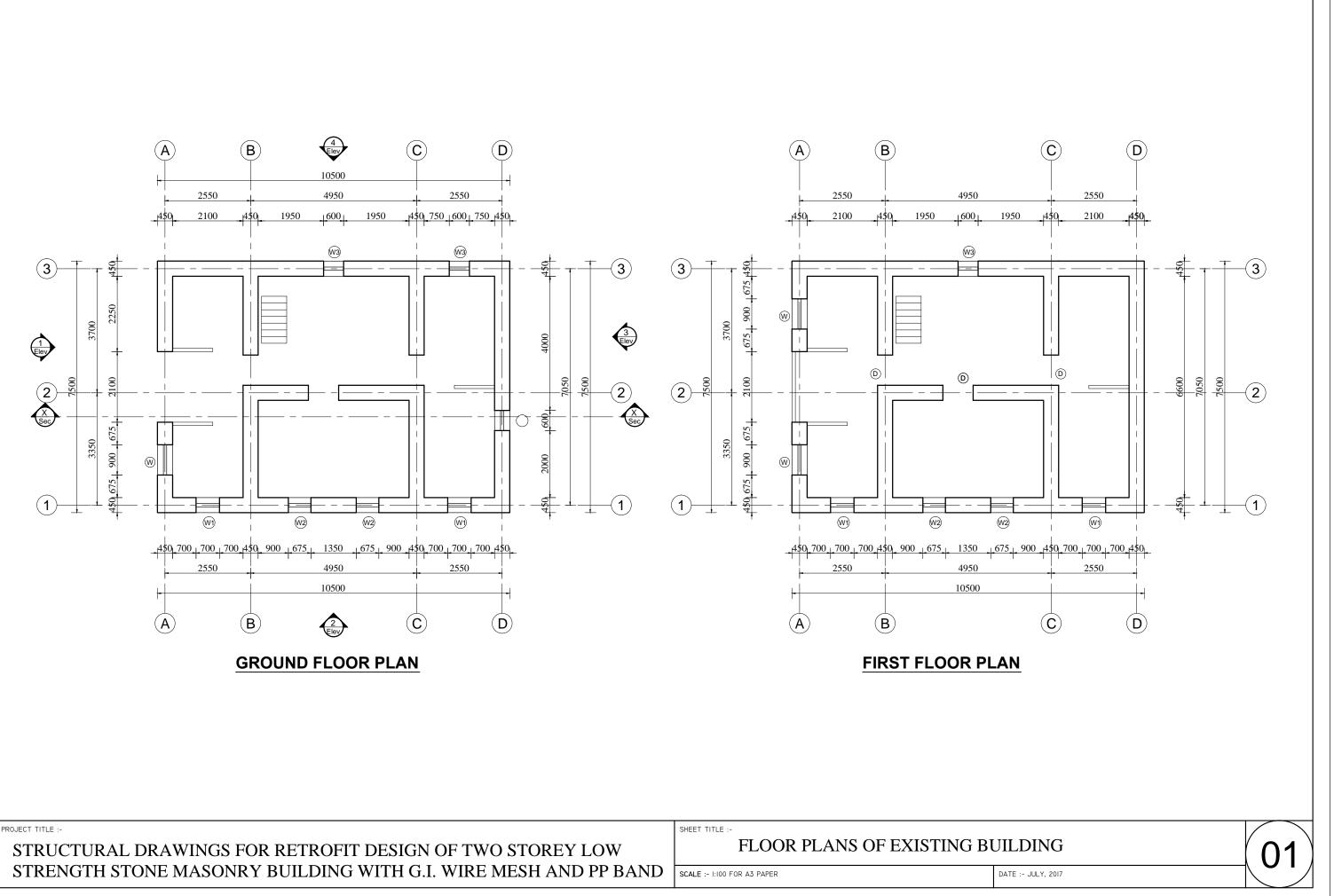
PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING WITH G.I. WIRE MESH AND PP BAND SCALE :- N/A

SHEET TITLE :-**COVER PAGE**

DATE :- JULY, 2017

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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING WITH G.I. WIRE MESH AND PP BAND

5400 (A)**(B**) 4950 1950 600 1950 \bigcirc 3--(3) 3 450 4\$0 _ SLOPE ROOF SLOPE ROOF SLOPE ROOF 2 2 2 2 9600 6600 7050 SLOPE ROOF 7500 SLOPE ROOF SLOPE ROOF -(2) 2 SLOPE ROOF SLOPE ROOF SLOPE ROOF (1)450 〔1〕 1 _ W2 (W2) 450 900 675 1350 675 900 450 4950 (\mathbf{A}) **(B**) 10500 **(B**) (\mathbf{C}) (D)**ROOF PLAN** (A)SECOND FLOOR PLAN

 (\mathbf{D})

 (\mathbf{C})

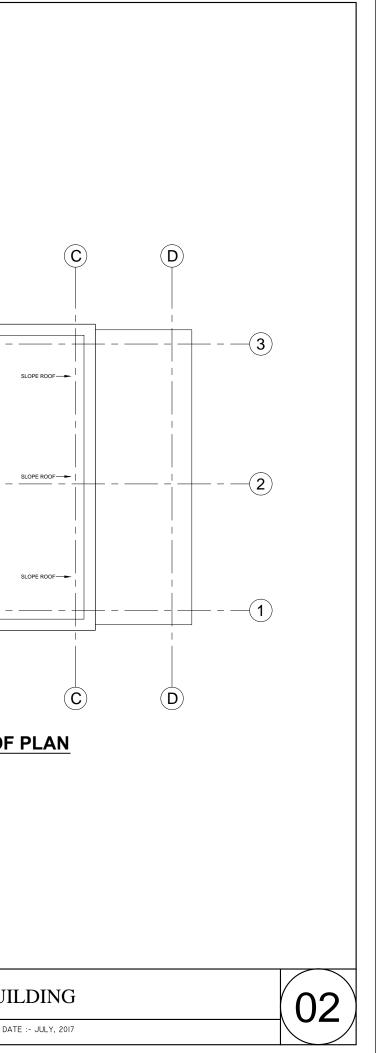
 (\mathbf{A})

 (\mathbf{B})

 PROJECT TITLE : SHEET TITLE :

 STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW
 FLOOR PLANS OF EXISTING BUILDING

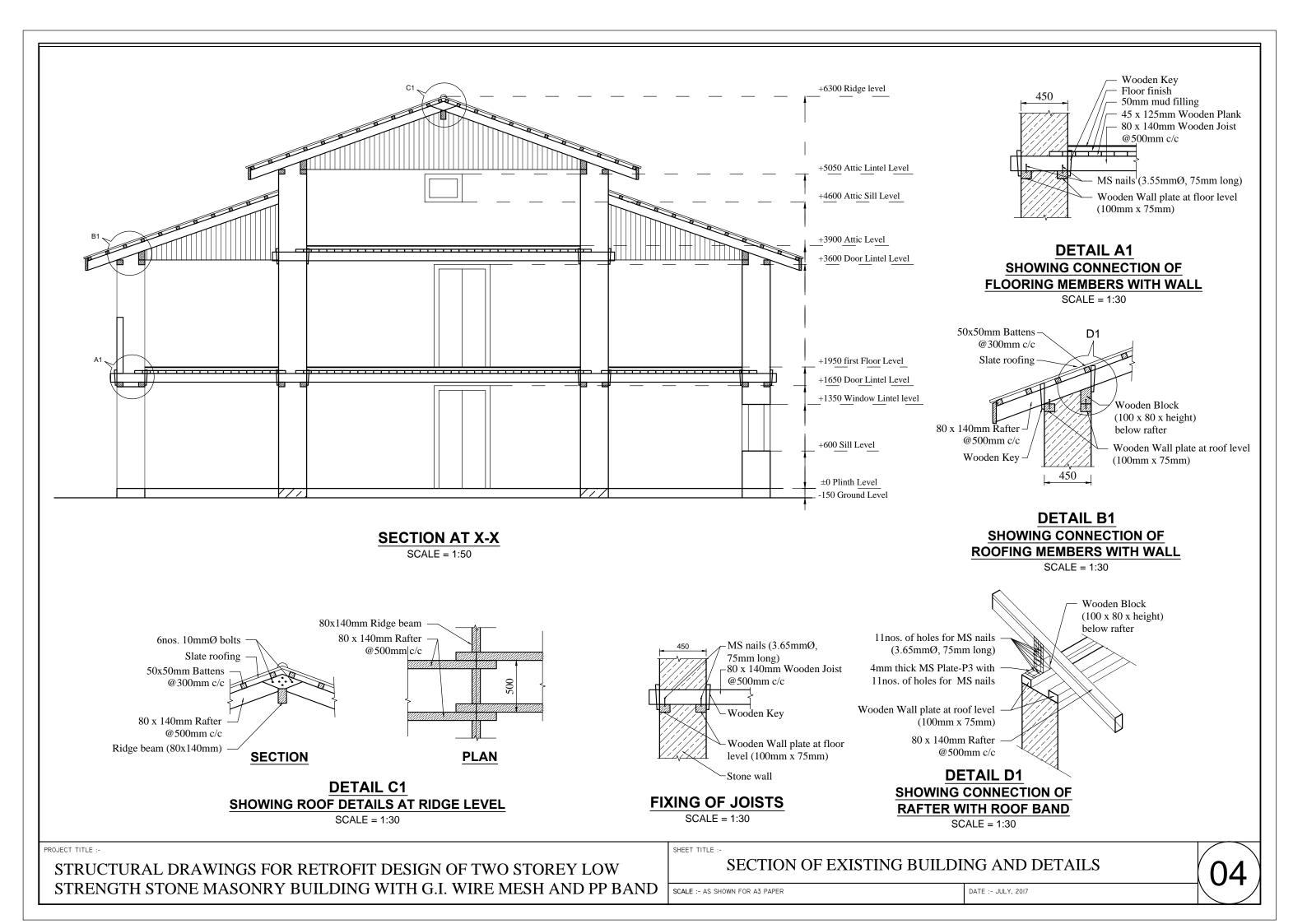
 STRENGTH STONE MASONRY BUILDING WITH G.I. WIRE MESH AND PP BAND
 SCALE :- 1:100 FOR A3 PAPER

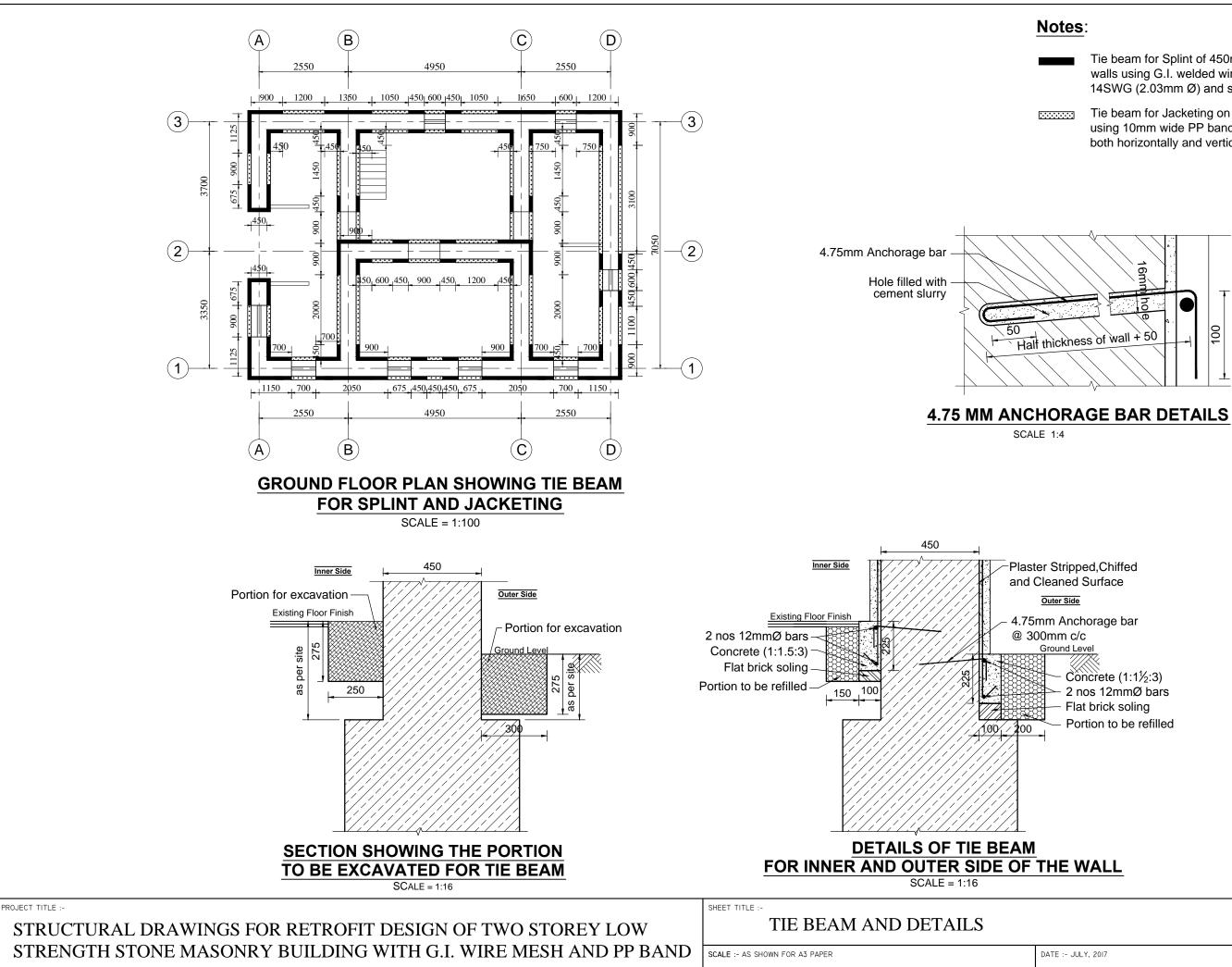


Slate roofin ate roofino **ELEVATION-3 ELEVATION-1** Light weight Gable wal Light weight Gable wal **ELEVATION-2 ELEVATION-4** PROJECT TITLE :-SHEET TITLE :-ELEVATIONS OF EXISTING BUILDING STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING WITH G.I. WIRE MESH AND PP BAND SCALE :- 1:100 FOR A3 PAPER

03

DATE :- JULY, 2017

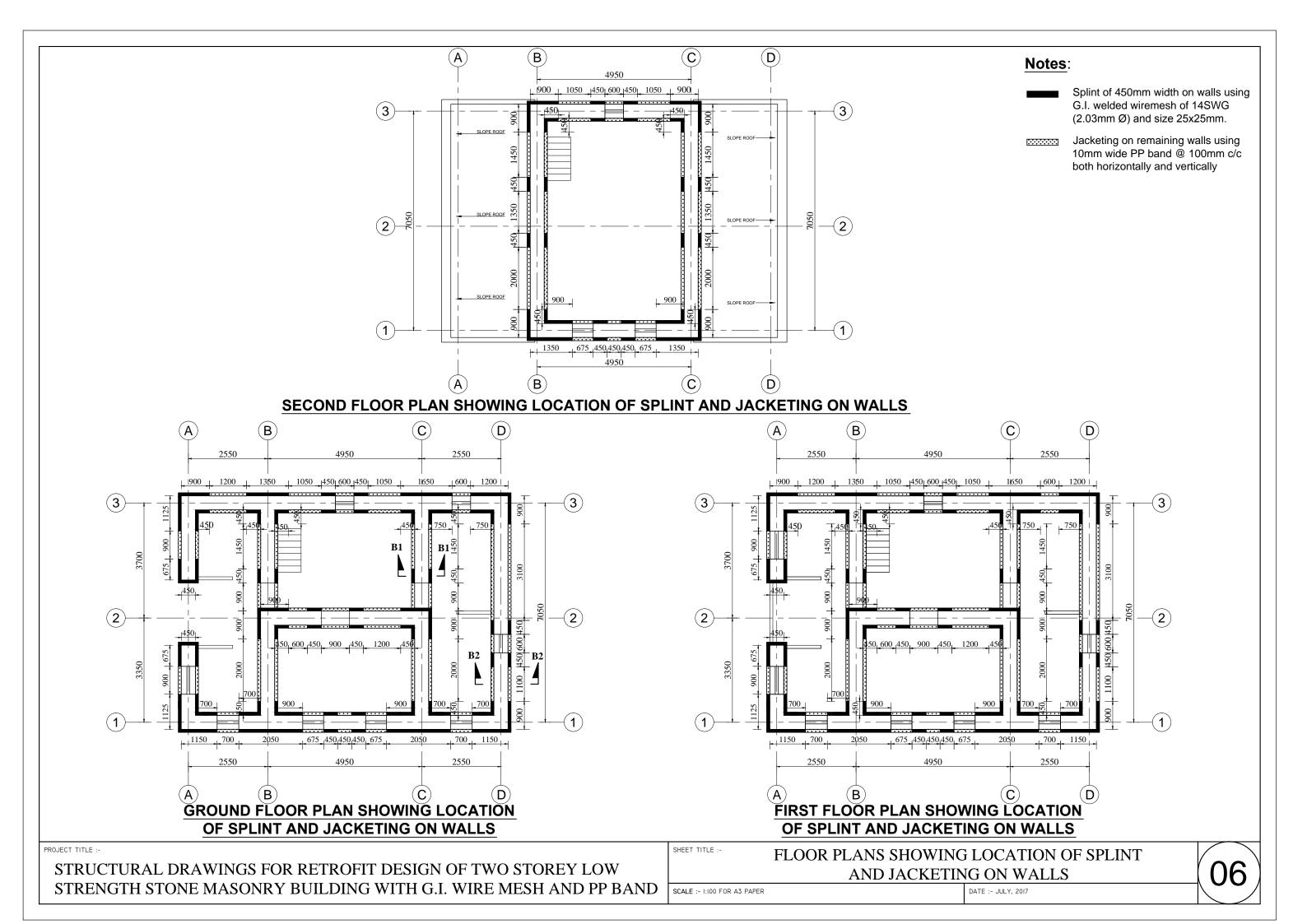


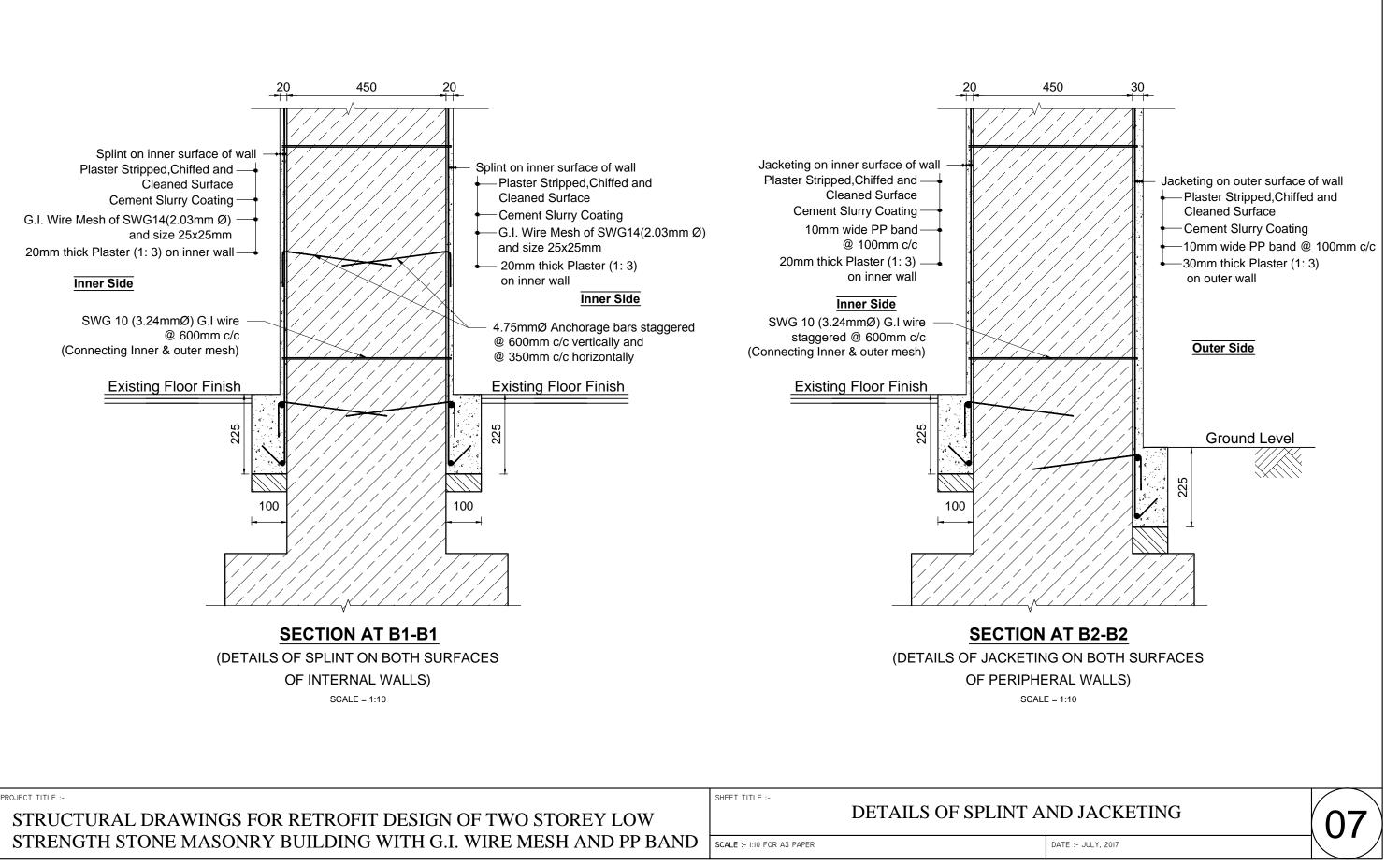


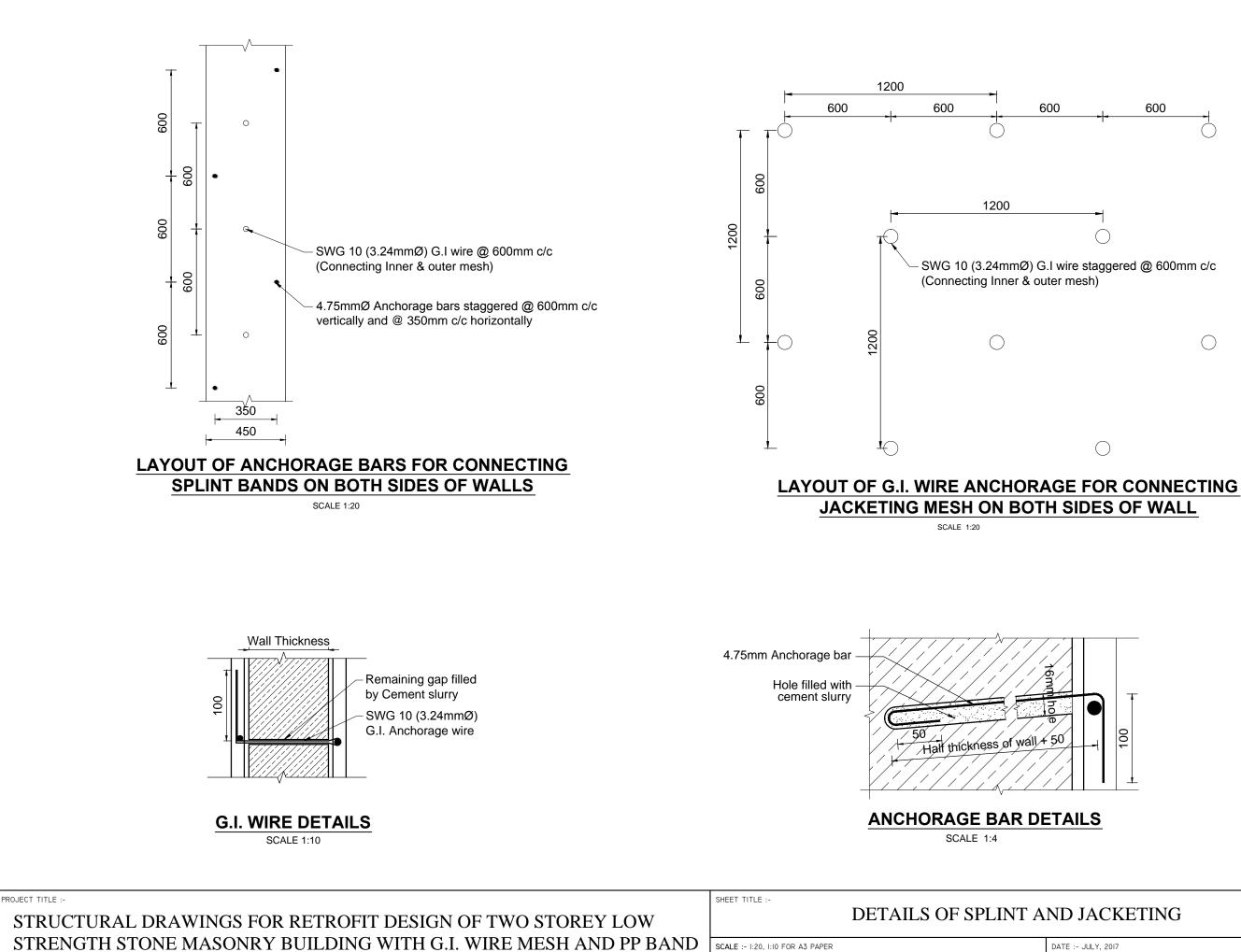
Tie beam for Splint of 450mm width on walls using G.I. welded wiremesh of 14SWG (2.03mm Ø) and size 25x25mm.

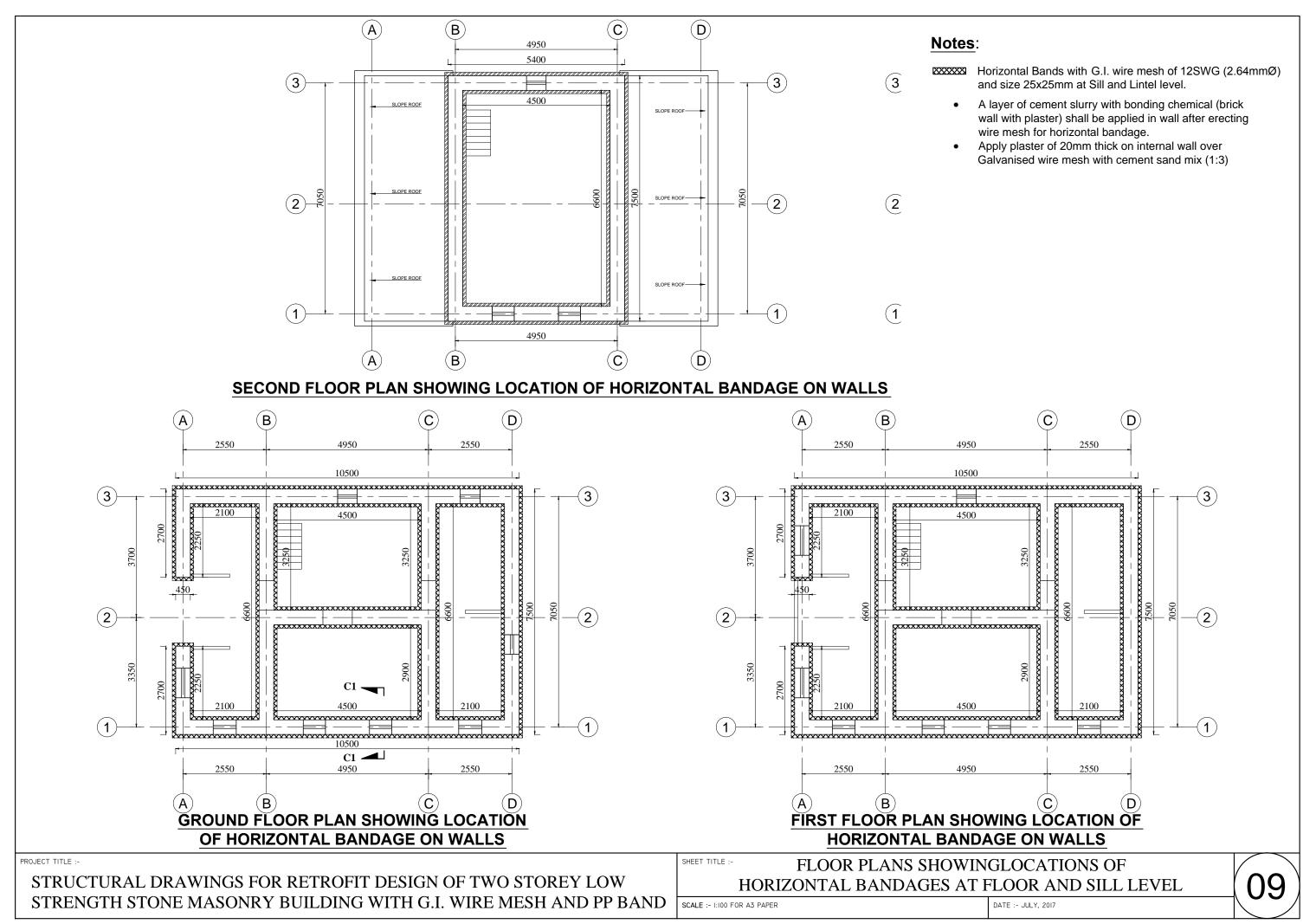
Tie beam for Jacketing on remaining walls using 10mm wide PP band @ 100mm c/c both horizontally and vertically

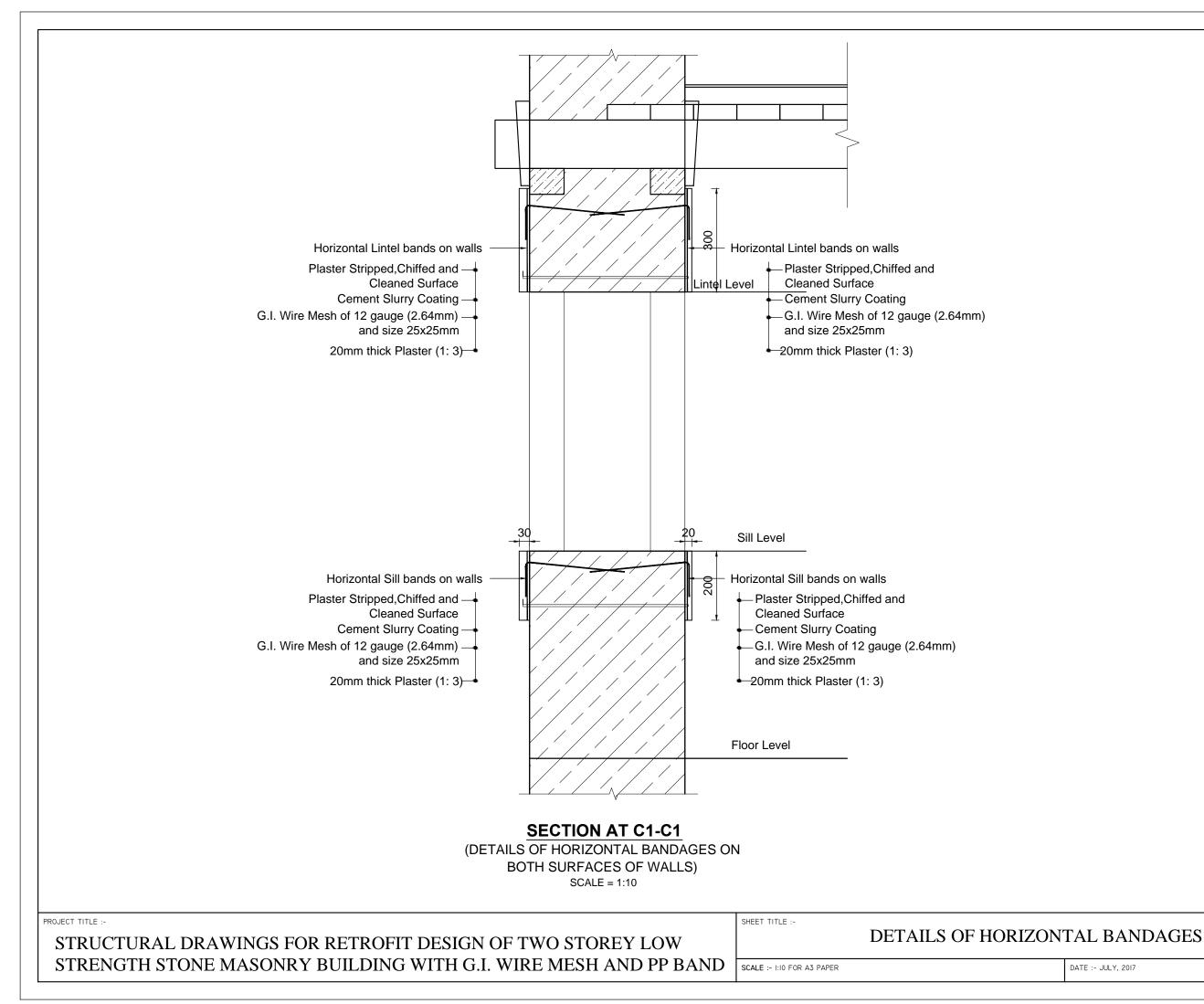
05





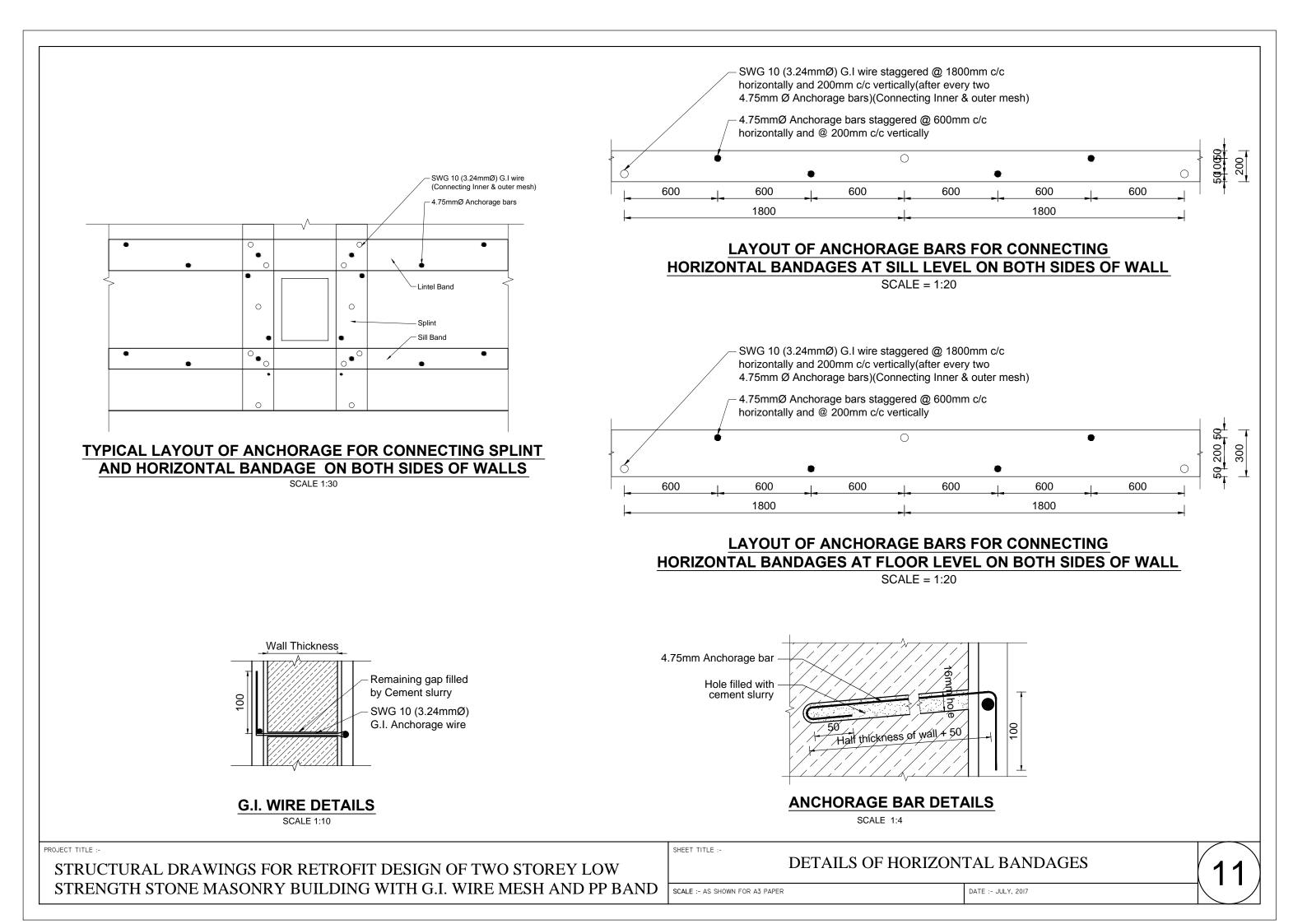


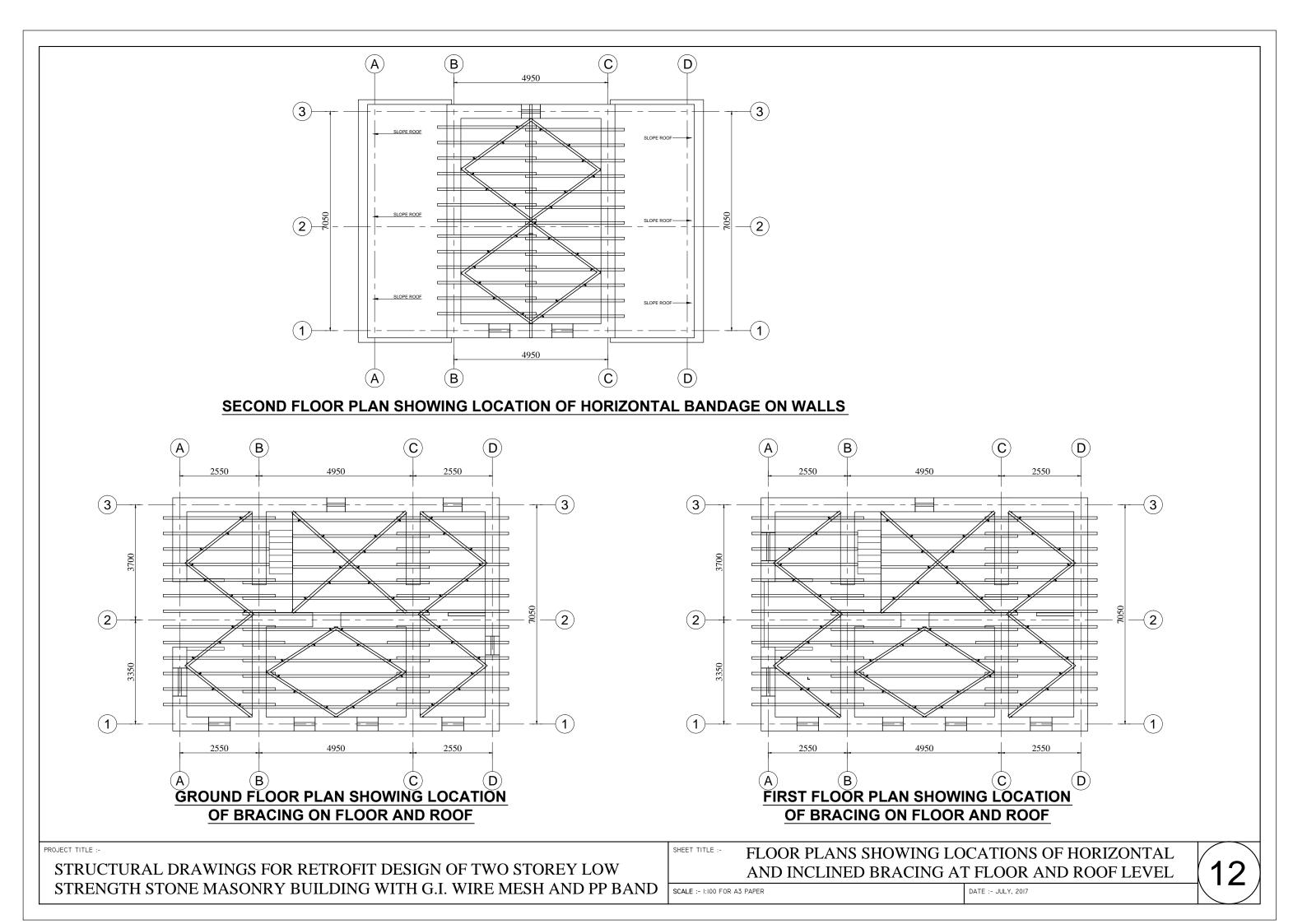


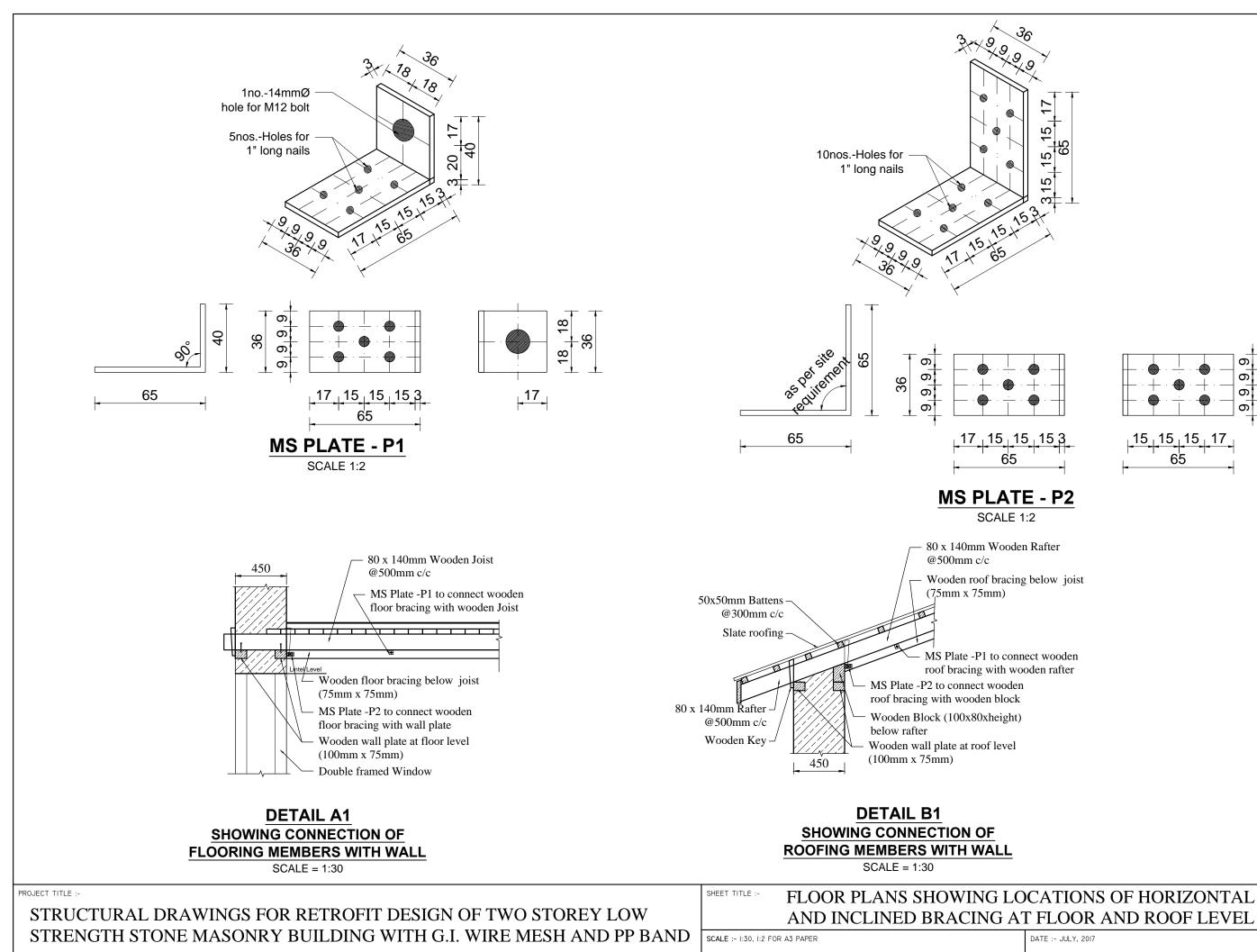


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DATE :- JULY, 2017

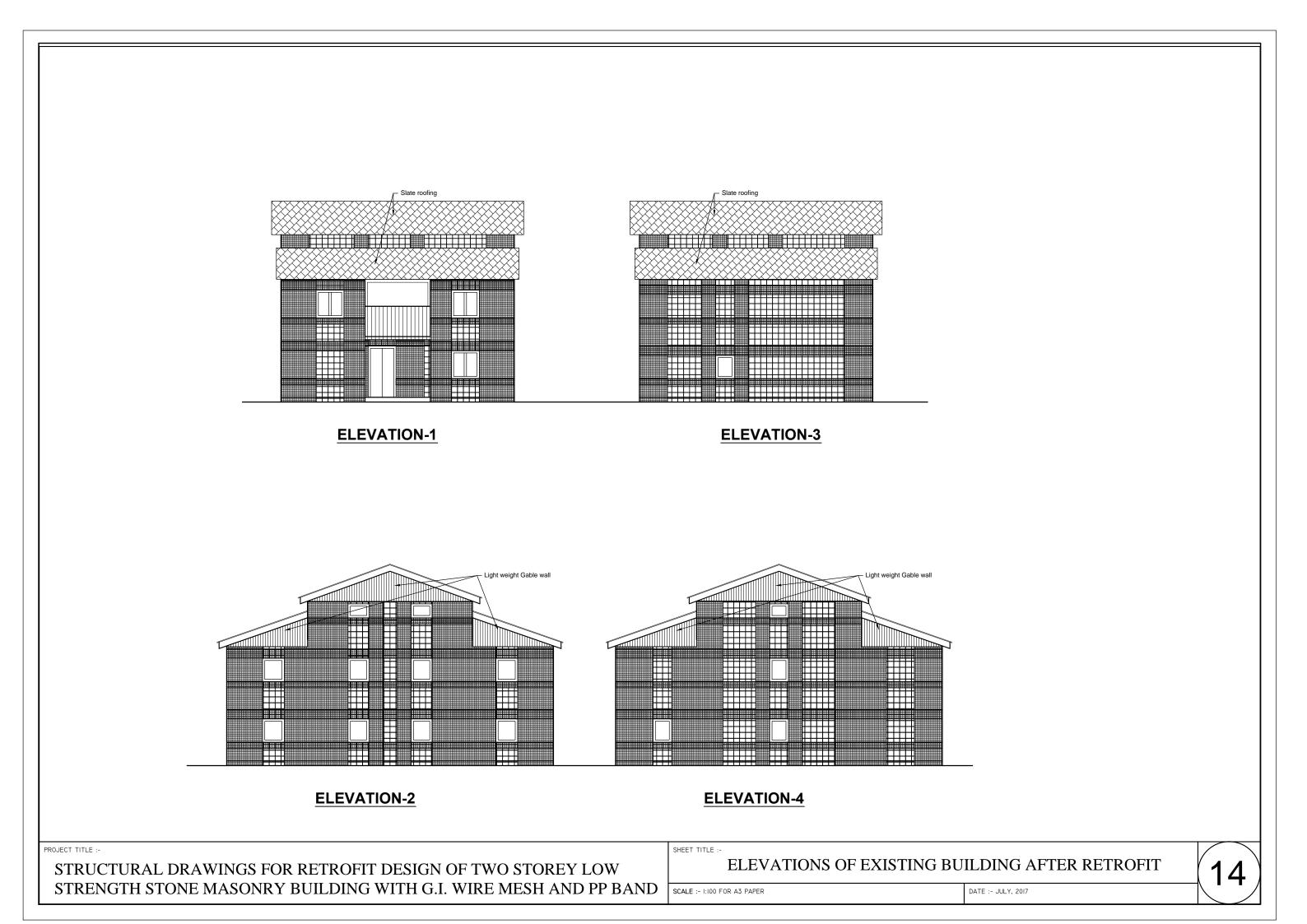






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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN ON TWO STOREY STONE MASONRY BUILDING IN MUD USING TIMBER AND G.I. WIREMESH

PROJECT TITLE :-

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING TIMBER AND G.I. WIRE MESH

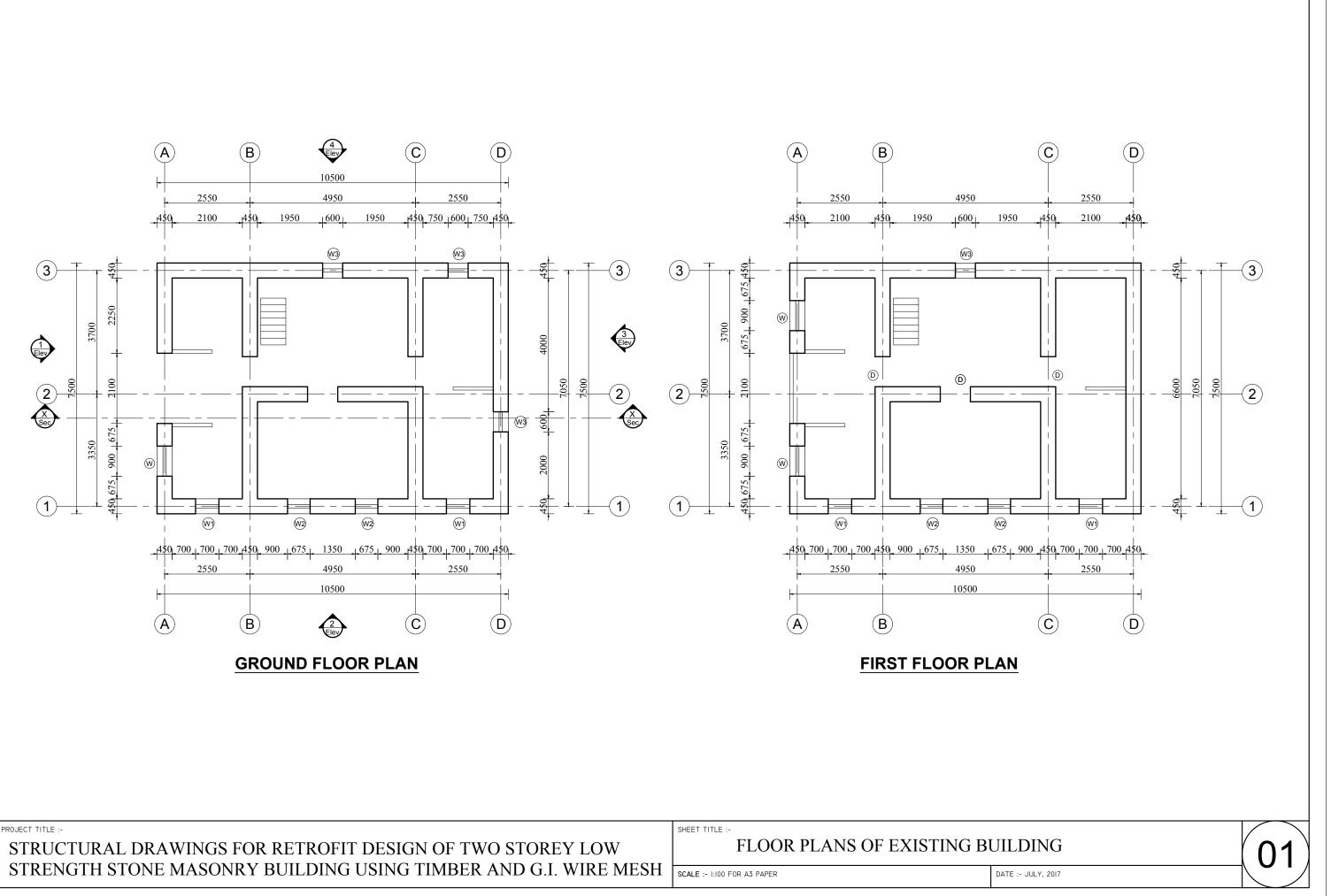
SHEET TITLE :-

COVER PAGE

DA

DATE :- JULY, 2017

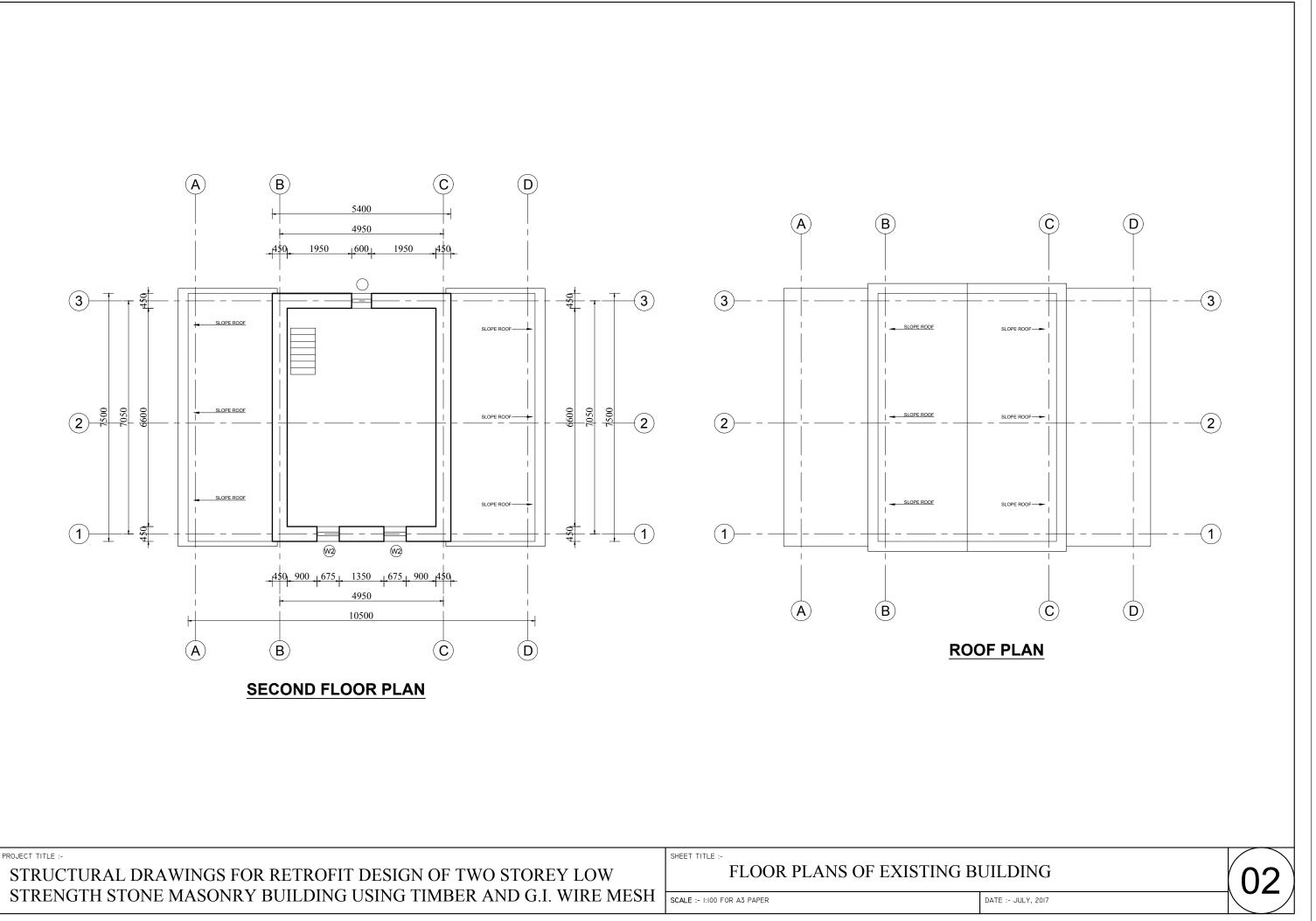
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STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING TIMBER AND G.I. WIRE MESH

STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW STRENGTH STONE MASONRY BUILDING USING TIMBER AND G.I. WIRE MESH SCALE :- 1:100 FOR A3 PAPER



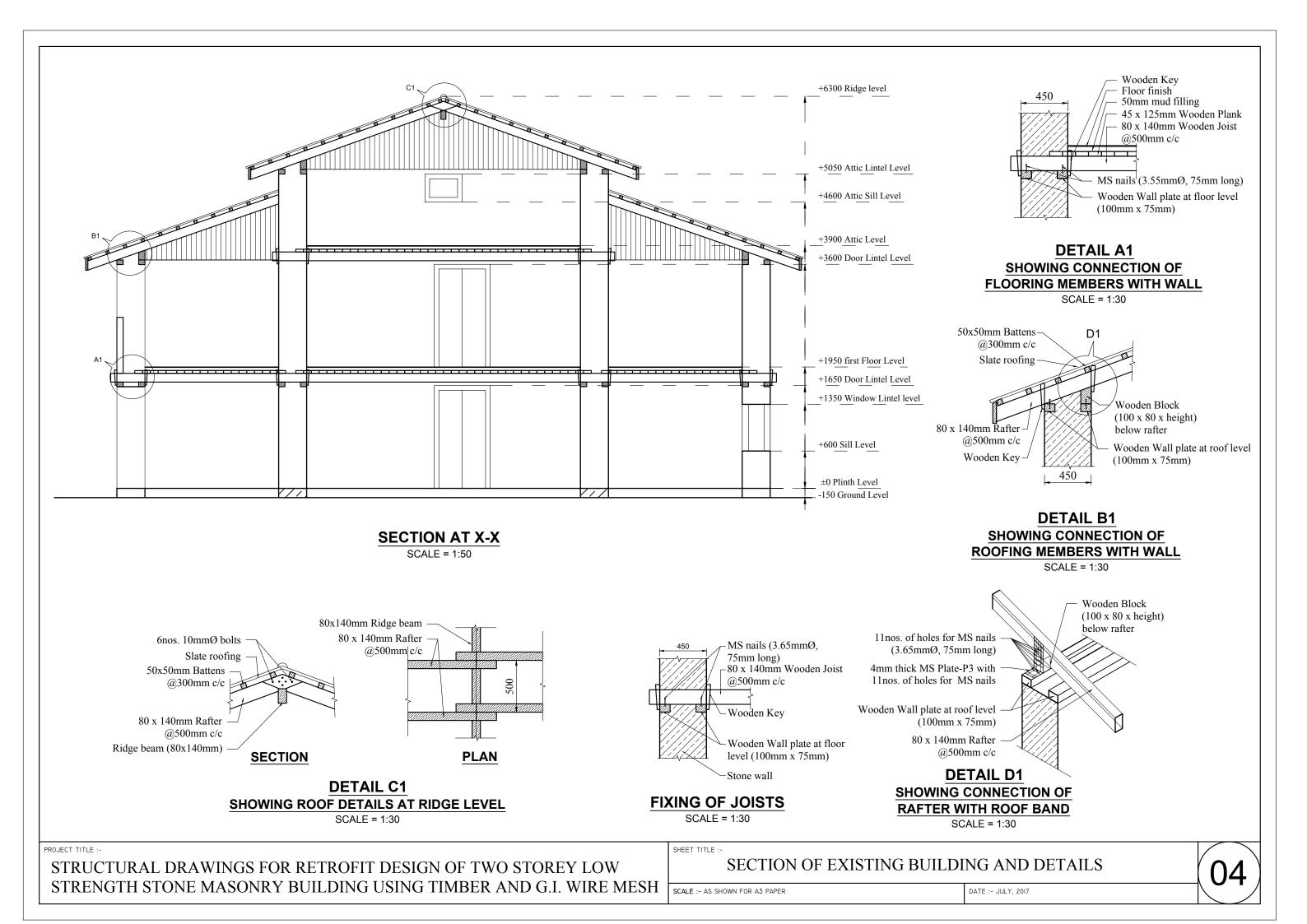


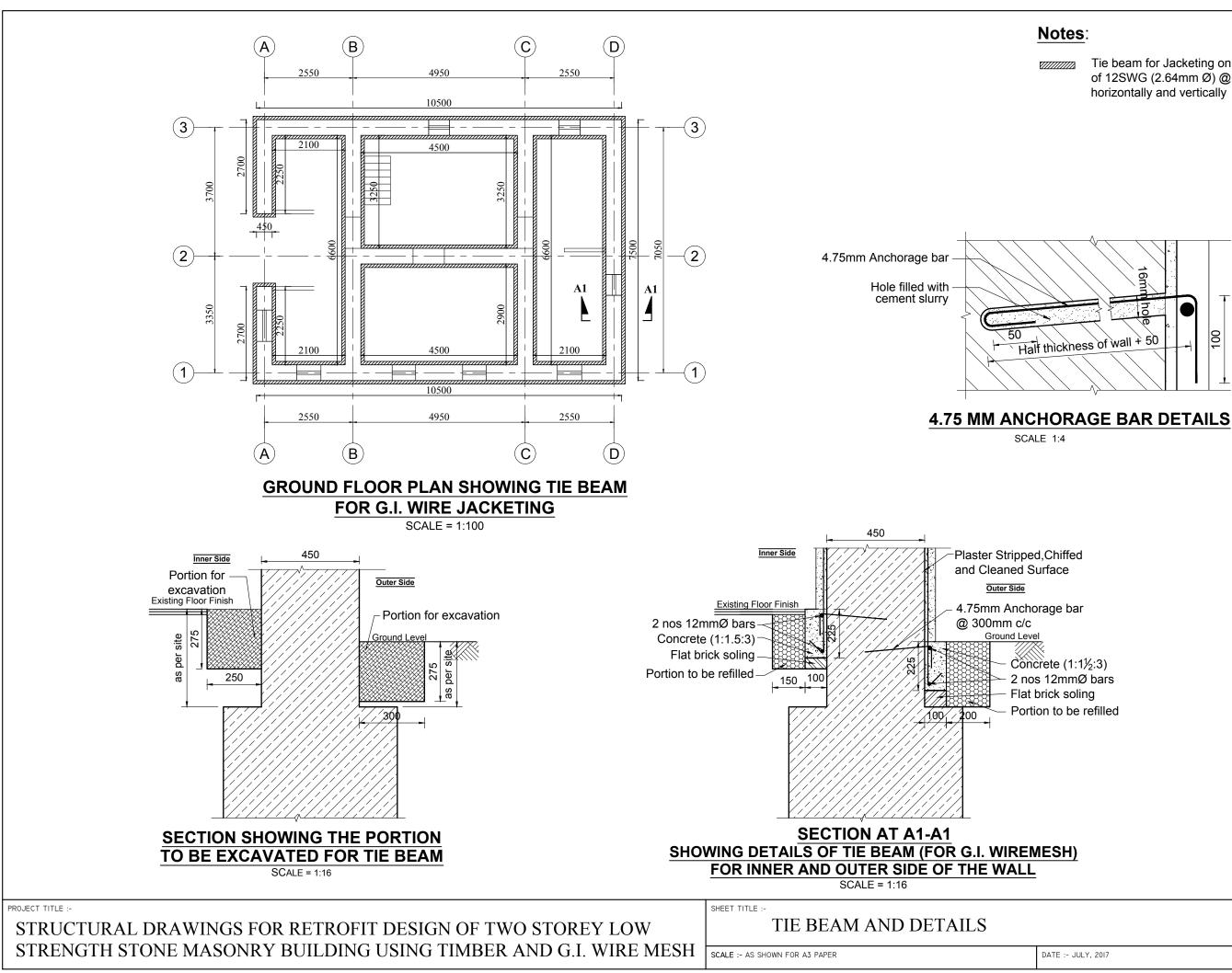
ELEVATION-1	ELEVATION-3
Light weight Gable wall	Light weight Gable wa

DING

03

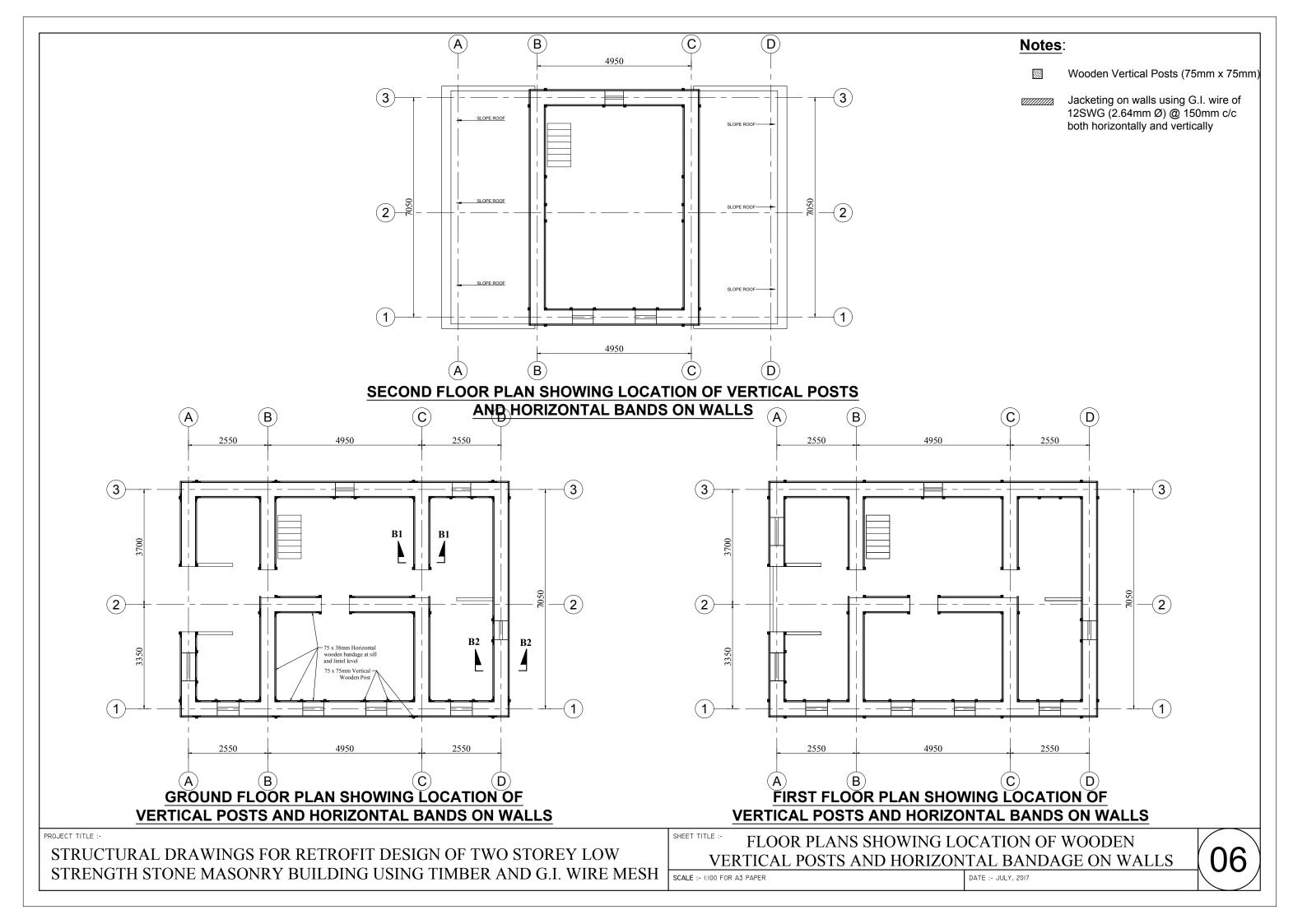
TE :- JULY, 2017

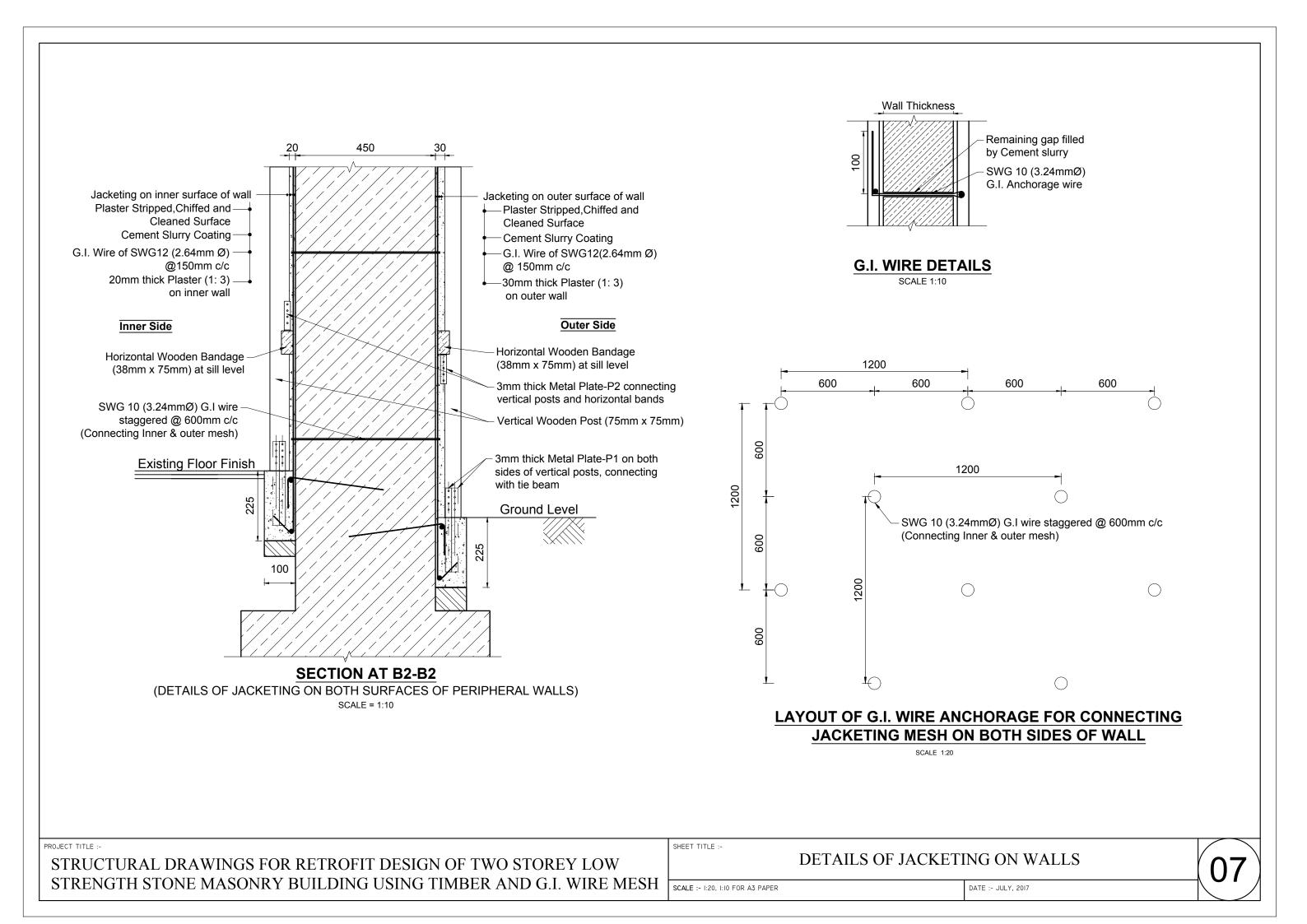


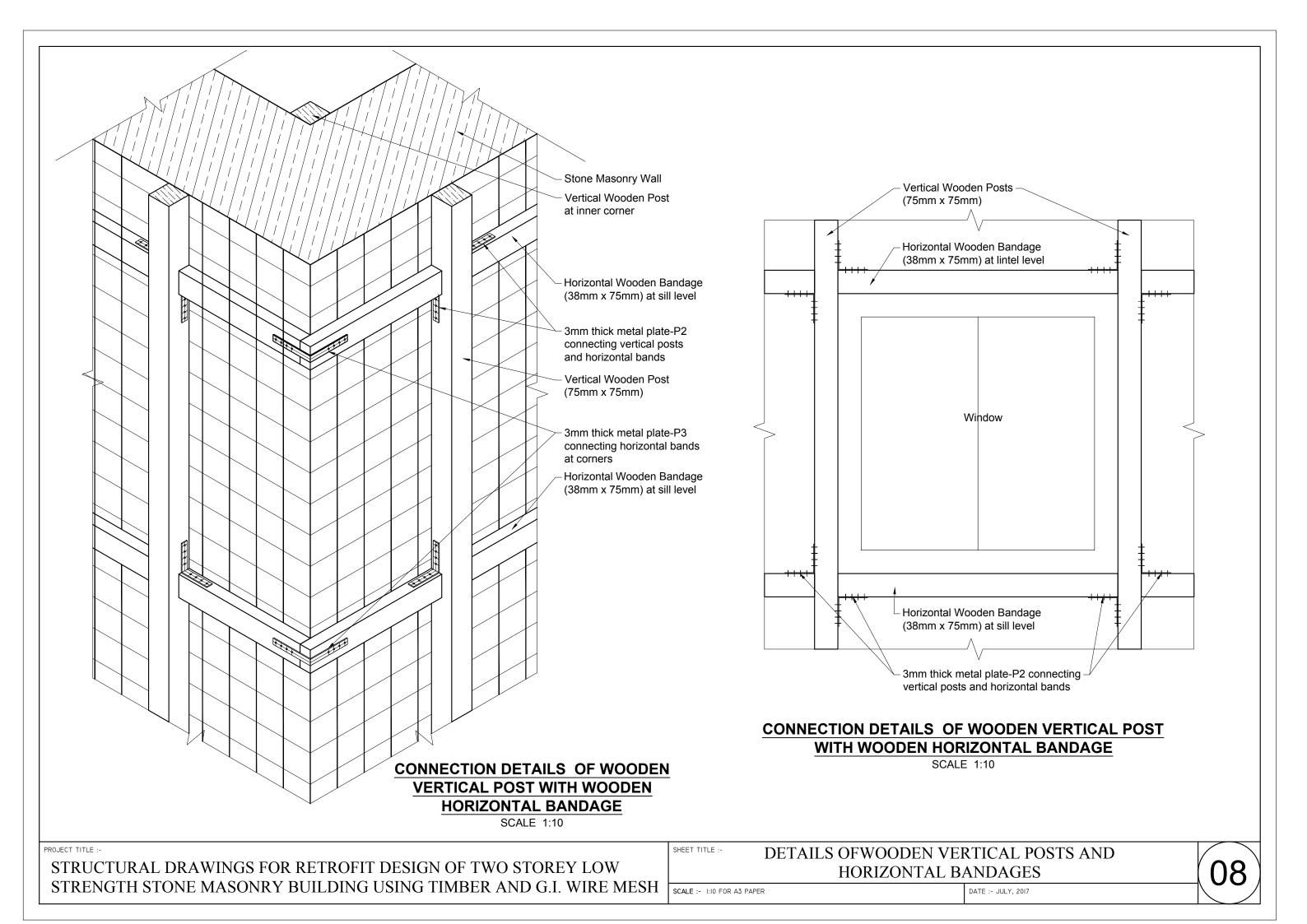


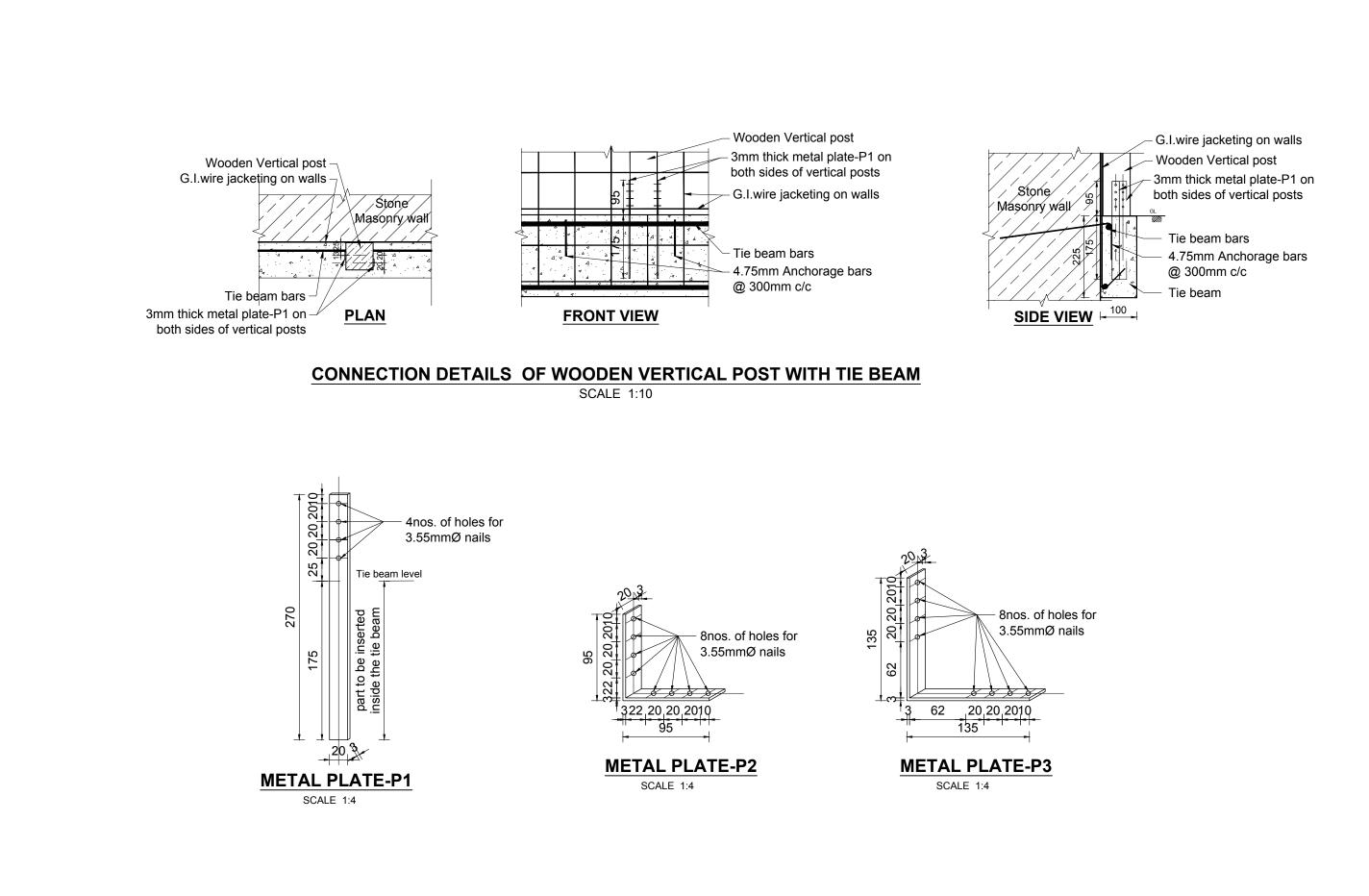
Tie beam for Jacketing on walls using G.I. wire of 12SWG (2.64mm Ø) @ 150mm c/c both

05









PROJECT TITLE :-	SHEET TITLE :- D	ETAILS OF WOODEN VER
STRUCTURAL DRAWINGS FOR RETROFIT DESIGN OF TWO STOREY LOW		HORIZONTAL BAN
STRENGTH STONE MASONRY BUILDING USING TIMBER AND G.I. WIRE MESH	SCALE :- 1:10, 1:4 FOR A3 PAPER	R DA

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:- JULY, 2017

